

AD-A050 315

COASTAL ENGINEERING RESEARCH CENTER FORT BELVOIR VA  
A SPATIALLY INTEGRATED NUMERICAL MODEL OF INLET HYDRAULICS.(U)

F/G 8/3

NOV 77 W N SEELIG, D L HARRIS

UNCLASSIFIED

CERC-6111-14

NL

1 OF 2

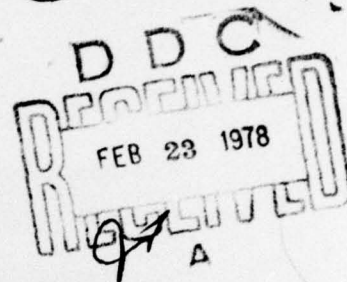
AD A050 315



AD A050315

# A Spatially Integrated Numerical Model of Inlet Hydraulics

(12)



by

William N. Seelig, D. Lee Harris, and Barry E. Herchenroder

AD No. \_\_\_\_\_  
DDC FILE COPY

GITI REPORT 14



November 1977

## GENERAL INVESTIGATION OF TIDAL INLETS

A Program of Research Conducted Jointly by  
U.S. Army Coastal Engineering Research Center, Fort Belvoir, Virginia  
U.S. Army Engineer Waterways Experiment Station, Vicksburg, Mississippi

Department of the Army  
Corps of Engineers

APPROVED FOR PUBLIC RELEASE; DISTRIBUTION UNLIMITED



**Reprint or republication of any of this material shall give appropriate credit to the U.S. Army Coastal Engineering Research Center.**

**Limited free distribution within the United States of single copies of this publication has been made by this Center. Additional copies are available from:**

*National Technical Information Service  
ATTN: Operations Division  
5285 Port Royal Road  
Springfield, Virginia 22151*

**Contents of this report are not to be used for advertising, publication, or promotional purposes. Citation of trade names does not constitute an official endorsement or approval of the use of such commercial products.**

**The findings in this report are not to be construed as an official Department of the Army position unless so designated by other authorized documents.**

**Cover Photo: Drum Inlet, North Carolina, 13 March 1962  
Courtesy of the U.S. Geological Survey**

UNCLASSIFIED

SECURITY CLASSIFICATION OF THIS PAGE (When Data Entered)

REPORT DOCUMENTATION PAGE		READ INSTRUCTIONS BEFORE COMPLETING FORM
1. REPORT NUMBER GITI Report 14	2. GOVT ACCESSION NO.	3. RECIPIENT'S CATALOG NUMBER
4. TITLE (and Subtitle) <b>6</b> A SPATIALLY INTEGRATED NUMERICAL MODEL OF INLET HYDRAULICS.	5. TYPE OF REPORT & PERIOD COVERED <b>9</b> Final Report.	6. PERFORMING ORG. REPORT NUMBER <b>14</b> CERC-GITI-14
7. AUTHOR(s) <b>10</b> William N./Seelig, D. Lee/Harris Barry E./Herchenroder	8. CONTRACT OR GRANT NUMBER(s)	
9. PERFORMING ORGANIZATION NAME AND ADDRESS Department of the Army Coastal Engineering Research Center (CERRE-CS) Kingman Building, Fort Belvoir, Virginia 22060	10. PROGRAM ELEMENT, PROJECT, TASK AREA & WORK UNIT NUMBERS F31019	
11. CONTROLLING OFFICE NAME AND ADDRESS Department of the Army Coastal Engineering Research Center Kingman Building, Fort Belvoir, Virginia 22060	12. REPORT DATE <b>11</b> November 1977	13. NUMBER OF PAGES 101 <b>12</b> 105 p.
14. MONITORING AGENCY NAME & ADDRESS (if different from Controlling Office)	15. SECURITY CLASS. (of this report) UNCLASSIFIED	15a. DECLASSIFICATION/DOWNGRADING SCHEDULE
16. DISTRIBUTION STATEMENT (of this Report)  Approved for public release, distribution unlimited.		
17. DISTRIBUTION STATEMENT (of the abstract entered in Block 20, if different from Report)		
18. SUPPLEMENTARY NOTES		
19. KEY WORDS (Continue on reverse side if necessary and identify by block number)  Computer program documentation      Tidal inlets Inlet hydraulics      Water level fluctuations Numerical model documentation		
20. ABSTRACT (Continue on reverse side if necessary and identify by block number) This report discusses the development of a simple numerical model for the prediction of coastal inlet velocities, discharge, and resulting bay level fluctuations. The model is a time-marching model that simultaneously solves the area-averaged momentum equation for the inlet and the continuity equation for the bay. It is assumed that the bay surface elevation remains horizontal as it rises and falls. At each time step the geometric and hydraulic factors describing the inlet-bay system are calculated by evaluating flow conditions (Continued)		

DD FORM 1 JAN 73 1473 EDITION OF 1 NOV 65 IS OBSOLETE

UNCLASSIFIED

SECURITY CLASSIFICATION OF THIS PAGE (When Data Entered)

037 050

✓



UNCLASSIFIED

SECURITY CLASSIFICATION OF THIS PAGE(When Data Entered)

throughout the inlet and by spatially integrating this information to determine coefficients of the first-order differential equations.

This model, which includes the important terms in the equation of motion, is flexible, easy and inexpensive to use, and gives a good estimate of the inlet-bay system hydraulics for various conditions. The model can be used for single or multiple inlets, bays, and seas.

This report includes the model theory and derivation, a FORTRAN computer program for solving the model equations, and instructions for use of the program. Examples are given to illustrate how the model may be used to predict coastal inlet response to astronomical tides, seiching, tsunamis, and storm surges.





## FOREWORD

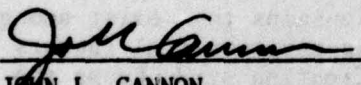
This report was prepared as one of a series of reports from the Corps of Engineers' General Investigation of Tidal Inlets (GITI). The GITI research program is under the technical surveillance of the Coastal Engineering Research Center (CERC), and is conducted by CERC, the U.S. Army Engineer Waterways Experiment Station (WES), other Government agencies, and private organizations. The model described in this report is the latest in a series of developments of numerical inlet hydraulic models beginning with Keulegan in 1967 and continuing with recent work at WES by Huval and others.

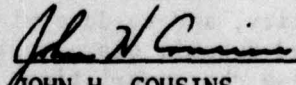
The report was prepared by William N. Seelig, Coastal Structures Branch, D. Lee Harris, Chief, Coastal Oceanography Branch, and Barry E. Herchenroder, Coastal Oceanography Branch, CERC. Development of this numerical model and report preparation were supervised by R.M. Sorensen, Chief, Coastal Structures Branch. Technical assistance was provided by C. Mason. Civilian members of the Coastal Engineering Research Board, Dean M.P. O'Brien, Prof. R.G. Dean, and Prof. R.L. Wiegel, reviewed this report.

Technical Directors of CERC and WES were T. Saville, Jr., and F.R. Brown, respectively.

Comments on this publication are invited.

Approved for publication in accordance with Public Law 166, 79th Congress, approved 31 July 1945, as supplemented by Public Law 172, 88th Congress, approved 7 November 1963.

  
JOHN L. CANNON  
Colonel, Corps of Engineers  
Commander and Director  
Waterways Experiment Station

  
JOHN H. COUSINS  
Colonel, Corps of Engineers  
Commander and Director  
Coastal Engineering Research Center

ACCESSION FOR	
NTIS	White Section <input checked="" type="checkbox"/>
AND	Buff Section <input type="checkbox"/>
UNCLASSIFIED	<input type="checkbox"/>
JUSTIFICATION	
BY	
DISTRIBUTION/AVAILABILITY CODES	
Dist.	AVAIL. and/or SPECIAL
A	

## PREFACE

1. The Corps of Engineers, through its Civil Works program, has sponsored, over the past 23 years, research into the behavior and characteristics of tidal inlets. The Corps' interest in tidal inlet research stems from its responsibilities for navigation, beach erosion prevention and control, and flood control. Tasked with the creation and maintenance of navigable U.S. waterways, the Corps dredges millions of cubic yards of material each year from tidal inlets that connect the ocean with bays, estuaries, and lagoons. Design and construction of navigation improvements to existing tidal inlets are an important part of the work of many Corps' offices. In some cases, design and construction of new inlets are required. Development of information concerning the hydraulic characteristics of inlets is important not only for navigation and inlet stability, but also because inlets, by allowing for the ingress of storm surges and egress of flood waters, play an important role in the flushing of bays and lagoons.

2. A research program, the General Investigation of Tidal Inlets (GITI), was developed to provide quantitative data for use in design of inlets and inlet improvements. It is designed to meet the following objectives:

To determine the effects of wave action, tidal flow, and related phenomena on inlet stability and on the hydraulic, geometric, and sedimentary characteristics of tidal inlets; to develop the knowledge necessary to design effective navigation improvements, new inlets, and sand transfer systems at existing tidal inlets; to evaluate the water transfer and flushing capability of tidal inlets; and to define the processes controlling inlet stability.

3. The GITI is divided into three major study areas: (a) inlet classification, (b) inlet hydraulics, and (c) inlet dynamics.

*a. Inlet Classification.* The objectives of the inlet classification study are to classify inlets according to their geometry, hydraulics, and stability, and to determine the relationships that exist among the geometric and dynamic characteristics and the environmental factors that control these characteristics. The classification study keeps the general investigation closely related to real inlets and produces an important inlet data base useful in documenting the characteristics of inlets.

*b. Inlet Hydraulics.* The objectives of the inlet hydraulics study are to define tide-generated flow regime and water level fluctuations in the vicinity of coastal inlets and to develop techniques for predicting these phenomena. The inlet hydraulics study is divided into three areas: (1) idealized inlet model study, (2) evaluation of state-of-the-art physical and numerical models, and (3) prototype inlet hydraulics.



(1) The Idealized Inlet Model. The objectives of this model study are to determine the effect of inlet configurations and structures on discharge, head loss and velocity distribution for a number of realistic inlet shapes and tide conditions. An initial set of tests in a trapezoidal inlet was conducted between 1967 and 1970. However, in order that subsequent inlet models are more representative of real inlets, a number of "idealized" models representing various inlet morphological classes are being developed and tested. The effects of jetties and wave action on the hydraulics are included in the study.

(2) Evaluation of State-of-the-Art Modeling Techniques. The objectives of this part of the inlet hydraulics study are to determine the usefulness and reliability of existing physical and numerical modeling techniques in predicting the hydraulic characteristics of inlet-bay systems, and to determine whether simple tests, performed rapidly and economically, are useful in the evaluation of proposed inlet improvements. Masonboro Inlet, North Carolina, was selected as the prototype inlet which would be used along with hydraulic and numerical models in the evaluation of existing techniques. In September 1969 a complete set of hydraulic and bathymetric data was collected at Masonboro Inlet. Construction of the fixed-bed physical model was initiated in 1969, and extensive tests have been performed since then. In addition, three existing numerical models were applied to predict the inlet's hydraulics. Extensive field data were collected at Masonboro Inlet in August 1974 for use in evaluating the capabilities of the physical and numerical models.

(3) Prototype Inlet Hydraulics. Field studies at a number of inlets are providing information on prototype inlet-bay tidal hydraulic relationships and the effects of friction, waves, tides, and inlet morphology on these relationships.

c. *Inlet Dynamics.* The basic objective of the inlet dynamics study is to investigate the interactions of tidal flow, inlet configuration, and wave action at tidal inlets as a guide to improvement of inlet channels and nearby shore protection works. The study is subdivided into four specific areas: (1) model materials evaluation, (2) movable-bed modeling evaluation, (3) reanalysis of a previous inlet model study, and (4) prototype inlet studies.

(1) Model Materials Evaluation. This evaluation was initiated in 1969 to provide data on the response of movable-bed model materials to waves and flow to allow selection of the optimum bed materials for inlet models.

(2) Movable-Bed Model Evaluation. The objective of this study is to evaluate the state-of-the-art of modeling techniques, in this case movable-bed inlet modeling. Since, in many cases, movable-bed modeling is the only tool available for predicting the response of an inlet to improvements, the capabilities and limitations of these models must be established.



4. This report describes a numerical model that can be used to predict inlet channel velocities and discharge as well as the resulting bay surface level oscillations for inlets responding to the tide and other long wave excitation. It has been developed as an easy to use, inexpensive method for a good "first look" analysis of inlet hydraulics. The need for this model arose during preliminary attempts to apply an earlier model developed under the GITI numerical model evaluation (see 3,b,(2) above), and described in Appendix 4 to GITI Report 6, "A Simplified (Lumped Parameter) Numerical Simulation" (Huval and Wintergerst, 1977). Although both models expand upon the Keulegan (1967) concept of a simple one-dimensional analysis, this model supersedes that of Appendix 4 since it provides more accurate results and is applicable to a wider variety of situations and conditions.

5. Included in this report are a derivation of the numerical model, documentation of the FORTRAN computer program used to apply the model, and example applications of the model to evaluate the hydraulic conditions at selected tidal and nontidal inlets.

## CONTENTS

	Page
CONVERSION FACTORS, U.S. CUSTOMARY TO METRIC (SI) . . . . .	9
SYMBOLS AND DEFINITIONS . . . . .	10
I INTRODUCTION. . . . .	13
II INLET HYDRAULICS. . . . .	13
III THE NUMERICAL MODEL . . . . .	15
1. Assumptions . . . . .	15
2. Derivation. . . . .	17
3. Weighting Functions . . . . .	23
IV USE OF THE COMPUTER PROGRAM . . . . .	25
1. General Conditions for Model Use. . . . .	25
2. Examples. . . . .	28
V SUMMARY AND CONCLUSIONS . . . . .	52
VI RECOMMENDATIONS . . . . .	54
LITERATURE CITED. . . . .	55

### APPENDIX

A WEIGHTING FUNCTIONS TO MINIMIZE FRICTION. . . . .	57
B CONSTRUCTING FLOW NETS. . . . .	59
C ALTERNATIVE DEVELOPMENT OF THE CONVECTIVE ACCELERATION TERM .	72
D COMPUTER PROGRAM (INLET) DOCUMENTATION. . . . .	75

### TABLE

Predicted Cabin Point Creek hydraulics. . . . .	52
---	----

### FIGURES

1 Inlet-bay system . . . . .	14
2 Inlet grid system. . . . .	20
3 Discharge of Brown Cedar Cut, Texas. . . . .	24
4 Pentwater, Michigan. . . . .	29



# CONTENTS

## FIGURES-Continued

	Page
5 Pentwater Inlet cross-sectional area. . . . .	30
6 Pentwater geometry used in the numerical model. . . . .	30
7 Spectral analysis of Pentwater Lake water levels. . . . .	31
8 Comparison of results from the Pentwater model application. . .	33
9 Pentwater model prediction of monochromatic forcing . . . . .	34
10 Response to long wave excitation at Pentwater, Michigan . . . .	35
11 Tsunami water level fluctuations in the sea . . . . .	37
12 Predicted inlet-bay system response to tsunami-generated seawater level fluctuations. . . . .	38
13 Masonboro Inlet, North Carolina, July 1974 . . . . .	39
14 Grid system for Masonboro Inlet, 1969 . . . . .	40
15 Predicted Masonboro Inlet hydraulics. . . . .	42
16 Prototype and predicted mean velocity in the throat of Masonboro Inlet. . . . .	43
17 Ebb velocity distribution . . . . .	44
18 Flood velocity distribution . . . . .	45
19 Indian River Inlet, Delaware. . . . .	46
20 Schematic of Indian River model geometry. . . . .	47
21 Indian River Inlet channel velocities and bay levels. . . . .	49
22 Cabin Point Creek, Virginia . . . . .	50
23 Cabin Point Creek sea and bay levels. . . . .	51
24 Effect of the width of inlet 2 on inlet and bay hydraulics, Cabin Point Creek, Virginia. . . . .	53



CONVERSION FACTORS, U.S. CUSTOMARY TO METRIC (SI)  
UNITS OF MEASUREMENT

U.S. customary units of measurement used in this report can be converted to metric (SI) units as follows:

Multiply	by	To obtain
inches	25.4	millimeters
	2.54	centimeters
square inches	6.452	square centimeters
cubic inches	16.39	cubic centimeters
feet	30.48	centimeters
	0.3048	meters
square feet	0.0929	square meters
cubic feet	0.0283	cubic meters
yards	0.9144	meters
square yards	0.836	square meters
cubic yards	0.7646	cubic meters
miles	1.6093	kilometers
square miles	259.0	hectares
knots	1.8532	kilometers per hour
acres	0.4047	hectares
foot-pounds	1.3558	newton meters
millibars	$1.0197 \times 10^{-3}$	kilograms per square centimeter
ounces	28.35	grams
pounds	453.6	grams
	0.4536	kilograms
ton, long	1.0160	metric tons
ton, short	0.9072	metric tons
degrees (angle)	0.1745	radians
Fahrenheit degrees	5/9	Celsius degrees or Kelvins <sup>1</sup>

<sup>1</sup>To obtain Celsius (C) temperature readings from Fahrenheit (F) readings, use formula:  $C = (5/9) (F - 32)$ .

To obtain Kelvin (K) readings, use formula:  $K = (5/9) (F - 32) + 273.15$ .

# SYMBOLS AND DEFINITIONS

Symbol in the report	Symbol in the computer program INLET	Definition
$A_c$	A(I,J)	cross-sectional area (square feet)
$A_b$	AB	cross-sectional area at the bay end of the inlet (square feet)
$A_s$	AS	cross-sectional area at the sea end of the inlet (square feet)
$A_{bay}$	ABAY	bay surface area (square feet)
$A_o$	ABY	bay surface area at datum (square feet)
—	AO	ocean forcing amplitude (feet)
B	B(I,J)	width (feet)
$C_i$	C(I)	flow resistance parameter
$C_1, C_2$	C1,C2	coefficients to evaluate Manning's bottom- friction factor, $n$ , where $n = C_1 - C_2 * D$
D	D(I,J)	total water depth (feet)
$d_{bay}$	—	depth of the bay (feet)
$d_{max}$	—	maximum water depth in the inlet (feet)
F	F	total inlet friction
$F_N$	—	minimum friction function
$F_x$	—	friction in the grid cells of a cross section
g	G	acceleration of gravity (feet per second squared)
h	H(I,J)	water level above datum (feet)
$h_B$	HB	water level at the bay end (feet)
$h_s$	HS	water level at the sea end (feet)
—	HOURS	model time (hours)



# SYMBOLS AND DEFINITIONS--Continued

IC	IC	number of channels in a grid
$I_g$	—	geometry integral (see text)
IS	IS	number of sections in a grid
$i,j$	I,J	subscripts indicating grid location
k	—	a constant in Manning's equation to maintain consistent units
$L_{bay}$	—	length of a bay (feet)
$L_{ij}$	L(I,J)	length of a grid (feet)
$L_{in}$	LENGTH	inlet length (feet)
M	NINLETS	number of inlets connecting the bay to the sea
m	NI	subscript identifying the inlet number
n	N(I,J)	Manning's bottom friction coefficient
—	NCYCLES	number of forcing water level cycles used in computation
—	NT	number of time steps used in computations
Q	Y(NI)	discharge of an inlet (cubic feet per second)
$Q_{ij}$	Q(I,J)	discharge of a grid (cubic feet per second)
$Q_{inflow}$	QINFLO	net discharge of water into the bay from sources other than the inlet (cubic feet per second)
$Q_T$	QT	total discharge of all inlets connecting a sea to a bay (cubic feet per second)
—	QINT(NI)	estimated inlet discharge at time the model starts (cubic feet per second)
R	—	remainder terms (neglected)
$T_F$	T	forcing wave period in the sea (hours or seconds)



# SYMBOLS AND DEFINITIONS--Continued

$T_H$	—	inlet-bay Helmholtz period (hours or seconds)
$T_H'$	THELM	inlet-bay Helmholtz period estimated by neglecting inlet friction (hours or seconds)
$t$	X	time of model operation (seconds)
—	THTF	ratio of the Helmholtz to forcing wave period
—	TIME	time (seconds)
$u$	$V(I,J)$	water velocity (feet per second)
$\bar{u}$	—	cross-sectional mean water velocity (feet per second)
—	VBAR	mean water velocity across the minimum area section (feet per second)
$W_{ij}$	$W(IJ,)$	grid weighting function for distributing flow throughout an inlet grid flow net
$x$	—	distance along a channel (feet)
$x_b$	—	bay limit
$x_s$	—	sea limit
$y, y_1, y_2$	—	distances perpendicular to the main axis of the inlet channel (feet)
$\Delta t$	DELTA	variable finite-difference time step (seconds)
$\beta$	BETA	bay surface area variation parameter relating bay area to bay water level where: $ABAY = ABY (1. + BETA * HB)$
—	ZETA	inlet side slope, $dZ/dy$ , where $Z$ is elevation
$\lambda$	—	Lagrangian multiplier
$\partial Q / \partial t$	DERBY	derivative of inlet discharge with respect to time (cubic feet per second squared)
$(\tau_{zx})_z$	—	component of the stress tensor representing the bottom stress

# A SPATIALLY INTEGRATED NUMERICAL MODEL OF INLET HYDRAULICS

by  
William N. Seelig, D. Lee Harris,  
and Barry E. Herchenroder

## I. INTRODUCTION

Quick, inexpensive estimates of inlet velocities and bay water surface levels for tidal or nontidal sea level fluctuations are needed in planning the design, construction, and maintenance of coastal inlets. Field data are often unavailable, and available data are often incomplete. In addition, hydraulic characteristics for proposed inlets are unavailable and must be predicted.

This study discusses these needs by developing a numerical model that can be used to estimate inlet velocities, discharge, and bay water levels as functions of time for a given time-dependent sea level fluctuation. The objective of this research was to develop a model that could be used to quantitatively predict hydraulics for as wide a range of conditions as possible, while being easy and inexpensive to use. Inlet hydraulics are predicted in this model by marching through time, simultaneously solving the momentum equation for flow in the inlet and the continuity equation relating the bay level to inlet discharge. The momentum equation is evaluated at each time step by integrating two-dimensional information into coefficients of the equation, using a weighting function (see App. A) and a flow net (see App. B) which systematically distribute flow throughout the inlet.

The advantages of this model are that it requires a minimum amount of input data, and it is easy and inexpensive to use as compared to a full two-dimensional model. Other features of the model are that it includes all potentially important terms developed from the three-dimensional momentum equations, and it allows for special situations. For example, water level fluctuations in the sea can be any function of time, the area of the bay can be a function of water level, and inflow into the bay from sources other than the inlet can be a function of time or water level of the bay. Also, the cross-sectional area of the inlet can be taken as a function of the local water depth. This model can be used to predict hydraulics for tidal or nontidal, single and multiple inlet systems. It assumes that the bay level "pumps" (i.e., rises and falls) at the same rate and phase throughout the bay.

## II. INLET HYDRAULICS

An inlet-bay system typically consists of a "sea" (e.g., ocean or lake) connected to a "bay" by one or more inlets (Fig. 1). Long waves in the sea (i.e., astronomical tides, seiches, storm surges, tsunamis, or other water level fluctuations) generate the primary hydraulic response in the inlet-bay system. The difference in water level between the bay and sea, caused by the sea forcing fluctuations, results in reversing



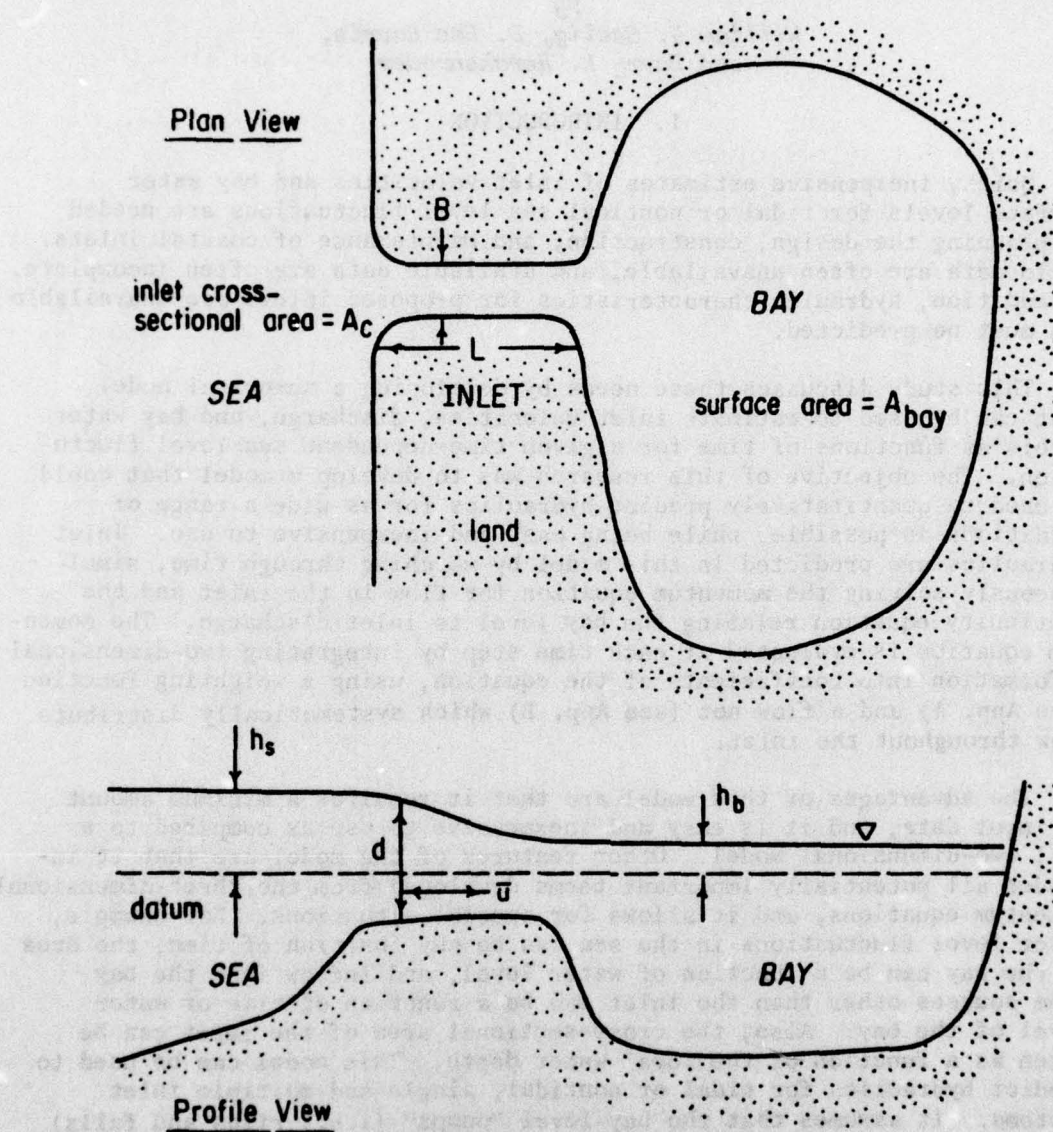


Figure 1. Inlet-bay system.

currents in the inlet which in turn cause the bay level to change. The bay level may also be influenced by inflow from and outflow to other sources (i.e., river discharge or evaporation) which may be a function of time or of the bay level. Additional factors such as wind stress, water density variations, and the earth's rotation may also affect the system.

The prediction of reversing currents in the inlet and bay level fluctuations requires a knowledge of the geometries of the inlets and bay, the water level fluctuations in the sea which force the inlet-bay system, and a model (analytical, numerical, or physical) to predict the system response. For most inlets and bays, the geometries can be measured in the field, obtained from dredging records, or evaluated from hydrographic charts. Water level fluctuations in the sea must be measured in the field, predicted from tide tables or physical or numerical hydrodynamic models.

The complete three-dimensional equations of motion that describe the hydrodynamics of the inlet-bay system (Fig. 1) are complicated. Rather than try to solve the complete equations, various investigators have simplified them and attempted to model only the factors that the investigators considered important in inlet hydraulics. Some models, such as physical distorted scale models and some two-dimensional vertically integrated numerical models, have been used to adequately predict inlet hydraulics for most engineering purposes (Harris and Bodine, 1977). However, operation of these models is relatively expensive, requires expert operators, and usually takes several months of effort to obtain the required results.

Simple analytical models such as those of Brown (1928) and Keulegan (1967), which lump important parameters into a single quantity, require a number of restrictive assumptions. O'Brien and Clark (1974) concluded: "The Keulegan approach and similar analyses of inlet hydraulics provide a useful qualitative framework for ordering data but they apply quantitatively only to small inlets and lagoons with simple inlet channel geometry."

More advanced lumped parameter models, such as those of King (1974) and Huval and Wintergerst (1977), have included additional terms in the equation of motion and have considered other effects to generalize the models. However, many of these more advanced lumped parameter models were based on earlier models and in the processes of expansion some generality was lost. Huval and Wintergerst (1977) and Harris and Bodine (1977) discuss previous models.

### III. THE NUMERICAL MODEL

#### 1. Assumptions.

This spatially integrated numerical model is based on the equations derived from the complete equations of motion by Harris and Bodine (1977). Assumptions made in the process of model derivation were:



- (a) Sea level is a specified function of time.
- (b) The bay water level remains horizontal. This means that the bay water level rises and falls at the same rate throughout the entire bay at each point in time. This occurs when the length of the long wave forcing the system is much longer in the bay than the longest axis of the bay.
- (c) The bay is connected to the sea by one or more inlets.
- (d) At least one inlet must continuously connect the bay to the sea. Some areas of inlets may go dry during the water level cycle, and one or more inlets may go dry as long as one inlet contains water.
- (e) Bay water surface area is a function of bay water level (or a function of time).
- (f) Inlet cross-sectional area is a function of local depth (or a function of time).
- (g) The local water level slope in the inlet is assumed to be linearly related to the local friction loss along the inlet between the sea and bay levels.
- (h) There is a water level drop along the inlet that is proportional to the unrecovered velocity head lost through turbulent eddy diffusion in the bay (floodflow) or sea (ebb flow). (Alternate schemes could be used.)
- (i) Storage of water in the inlet is negligible. This means that the flow into the inlet is equal to the flow exiting the inlet at any time. In addition, the volume of water stored in the inlet between high and low water should be small compared to the tidal prism. This is generally the case if the surface area of the bay is much larger than the surface area of the inlet.
- (j) Wind stress on the inlet and bay surfaces is negligible. This means that the model is most useful for cases when the wind is light, has a short duration, or has a short fetch over the bay.
- (k) Water has constant properties throughout the inlet and bay. No attempt has been made to model saltwater intrusion or other density gradient effects.
- (l) Radiation stress (the interaction with wind waves) is neglected.
- (m) Coriolis effects are neglected.

If the characteristics of the inlet-bay systems modeled are not consistent with these assumptions, model results may be in error.

## 2. Derivation.

The derivation of this model begins with the one-dimensional equation of motion as derived by Harris and Bodine (1977):

$$\frac{\partial \bar{u}}{\partial t} + \frac{1}{2} \frac{\partial}{\partial x} \bar{u}^2 + g \frac{\partial h}{\partial x} + \frac{1}{A_C} \int_{y_1}^{y_2} (\tau_{zx})_Z dy = R \quad (1)$$

where

- $\bar{u}$  = cross-sectional mean water velocity in the inlet  
(positive on floodflow)
- $t$  = time
- $x$  = distance along the main axis of the inlet
- $h$  = water level above some datum
- $g$  = acceleration due to gravity
- $A_C$  = inlet cross-sectional flow area at  $x$
- $(\tau_{zx})_Z$  = component of the stress tensor at the bottom of the inlet in the direction of the main axis of the inlet
- $R$  = remainder terms, which are neglected in this model ( $R=0$ ). This means that the water level is taken as constant at each inlet cross section. See Harris and Bodine (1977) for a discussion of these neglected terms.

The first term on the left of equation (1) is the temporal acceleration, the second term is the convective or advective acceleration, the third term is the slope of water surface along the inlet, and the fourth term is the bottom stress.

To obtain a simplified equation, the expressions in equation (1) are integrated over the length of the inlet between the sea and the bay, where  $x_s$  and  $x_b$  are the respective limits:



$$\begin{aligned}
& \int_{x_s}^{x_b} \frac{\partial \bar{u}}{\partial t} dx + \int_{x_s}^{x_b} \frac{1}{2} \frac{\partial \bar{u}^2}{\partial x} dx + \int_{x_s}^{x_b} g \frac{\partial h}{\partial x} dx \\
& + \int_{x_s}^{x_b} \frac{1}{A_c} \int_{y_1}^{y_2} (\tau_{zx})_z dy dx = 0 .
\end{aligned} \tag{2}$$

Carrying out some of the integrations and rearranging, equation (2) becomes

$$\begin{aligned}
& \frac{\partial}{\partial t} \int_{x_s}^{x_b} \bar{u} dx + \frac{1}{2} [(\bar{u}_b)^2 - (\bar{u}_s)^2] \\
& + g [h_b - h_s] + \int_{x_s}^{x_b} \frac{1}{A_c} \int_{y_1}^{y_2} (\tau_{zx})_z dy dx = 0 .
\end{aligned} \tag{3}$$

In equation (3), terms involving  $\partial x_b / \partial t$  and  $\partial x_s / \partial t$  have been set to zero since  $x_b$  and  $x_s$  are taken to be independent of time.

From continuity the cross-sectional mean inlet water velocity is equal to the inlet discharge,  $Q$ , divided by the inlet cross-sectional area,  $A_c$ :

$$\bar{u} = Q/A_c . \tag{4}$$

Substituting equation (4) and using the product rule for integration, the first term on the left of equation (3) can be integrated to yield:

$$\int_{x_s}^{x_b} \frac{\partial \left( \frac{Q}{A_c} \right)}{\partial t} dx = \frac{\partial Q}{\partial t} \int_{x_s}^{x_b} \frac{dx}{A_c} + Q \frac{\partial}{\partial t} \left( \int_{x_s}^{x_b} \frac{dx}{A_c} \right), \tag{5}$$

where the second part of the equation,

$$\frac{\partial}{\partial t} \left( \int_{x_g}^{x_b} \frac{dx}{A_c} \right),$$

is taken as zero because channel storage terms are neglected.

After substitution of equation (4), the second and third terms on the left of equation (3) are

$$\frac{1}{2} \left( \frac{1}{A_b^2} - \frac{1}{A_g^2} \right) Q^2 + g(h_b - h_g) \quad (6)$$

where  $A_b$  and  $A_g$  are the cross-sectional areas of the inlet at the bay and sea ends of the inlet, and  $h_b$  and  $h_g$  are the water levels in the bay and sea referenced to a common datum. The convective acceleration, the left term of equation (6), may also be expressed in terms of empirical loss coefficients (see App. C).

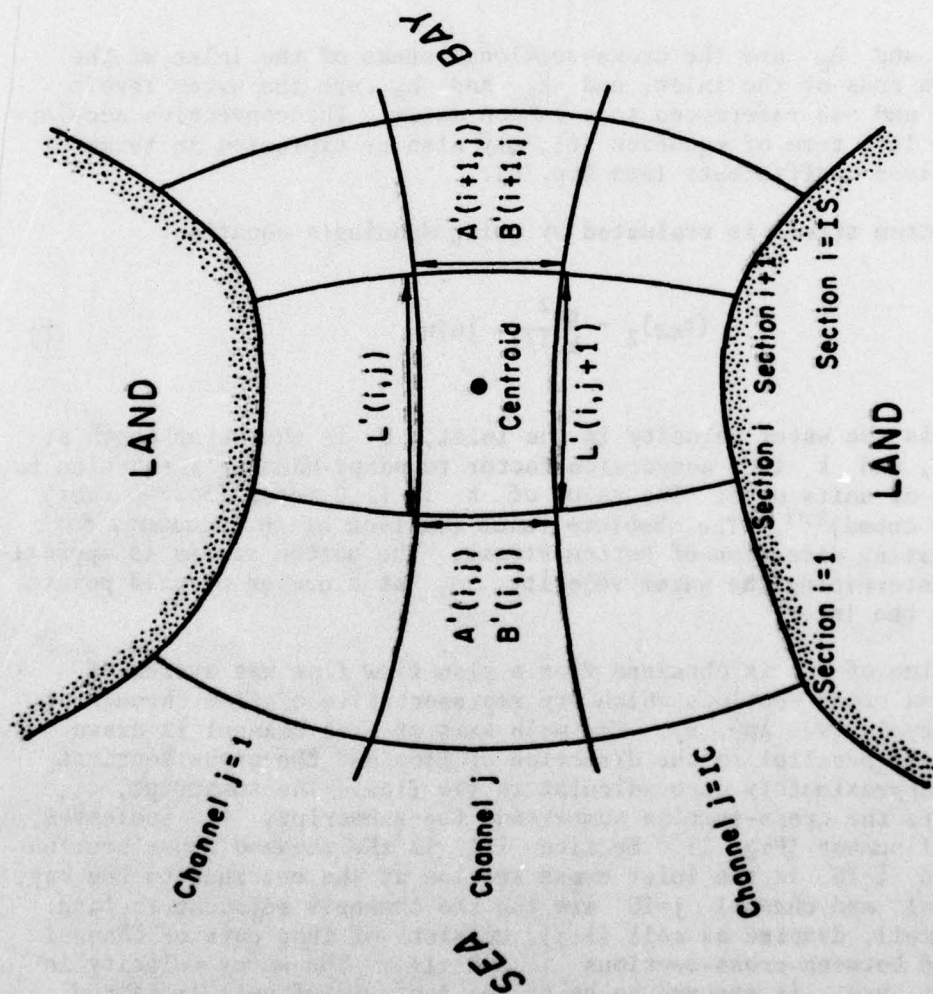
The bottom stress is evaluated by using Manning's equation:

$$(\tau_{zx})_z = \frac{gn^2}{kD^{1/3}} |u|u \quad (7)$$

where  $u$  is the water velocity in the inlet,  $D$  is the water depth at that point, and  $k$  is a conversion factor to adapt Manning's equation to the system of units used. The value of  $k$  is [1.0 meter (3.2808 feet) per second cubed]<sup>2/3</sup>. The absolute value function of  $u$  accounts for the alternating direction of bottom stress. The bottom stress is approximated by determining the water velocity,  $u$ , at a number of grid points throughout the inlet.

The value of  $u$  is obtained from a plan view flow net system of channels and cross sections which are representative of flow throughout the tidal cycle (see App. B). The main axis of each channel is drawn approximately parallel to the direction of flow and the cross sections are drawn approximately perpendicular to the flow. The subscript,  $i$ , then denotes the cross-section number and the subscript,  $j$ , indicates the channel number (Fig. 2). Section  $i=1$  is the seaward cross section and section  $i=IS$  is the inlet cross section at the entrance to the bay. Channel  $j=1$  and channel  $j=IC$  are the two channels adjacent to land. A typical cell, denoted as cell  $(i,j)$ , consists of that part of channel  $j$  situated between cross-sections  $i$  and  $i+1$ . The water velocity in cell  $(i,j)$ ,  $u_{ij}$ , is assumed to be at the centroid of cell  $(i,j)$  and to act parallel to the axis of channel  $j$  (Fig. 2).





Cell characteristics

$$A(i,j) = \frac{A'(i,j) + A'(i+1,j)}{2}$$

$$B(i,j) = \frac{B'(i,j) + B'(i+1,j)}{2}$$

$$D(i,j) = A(i,j) / B(i,j)$$

$$L(i,j) = \frac{L'(i,j) + L'(i,j+1)}{2}$$

are applied at the cell centroid

Figure 2. Inlet grid system.

A weighting function,  $W_{ij}$ , is then used to determine what fraction of the total inlet flow,  $Q$ , passes through a grid cell  $(i,j)$  at an instant in time:

$$Q_{ij} = W_{ij} Q \quad (8)$$

where  $Q_{ij}$  is the discharge in cell  $(i,j)$ . Various weighting functions are discussed later in this section.

The mean water velocity in cell  $(i,j)$ ,  $u_{ij}$ , is taken equal to the discharge in the cell divided by the mean cross-sectional area of the cell perpendicular to flow,  $A_{ij}$ ; i.e.,

$$u_{ij} = W_{ij} Q / A_{ij} \quad (9)$$

Substituting equations (7) and (9) into the fourth term of equation (3) and integrating over  $x$  and  $y$  give the total bottom friction,  $F$ :

$$F = \sum_{i=1}^{IS-1} \frac{1}{\sum_{j=1}^{IC} (A_{ij})} \sum_{j=1}^{IC} \frac{g(n_{ij})^2 |W_{ij} Q| W_{ij} Q B_{ij} L_{ij}}{k (D_{ij})^{1/3} A_{ij}^2} \quad (10)$$

where  $n_{ij}$  is the Manning's coefficient of friction for each grid cell,  $D_{ij}$  is the mean instantaneous water depth in a cell, and  $B_{ij}$  and  $L_{ij}$  are the mean cell width and length, respectively. All of the cell parameters are taken to apply to the centroid of the cell (Fig. 2).

Let  $I_g$  be a geometry integral defined for convenience as

$$I_g = \frac{1}{\int_{x_g}^{x_b} \frac{dx}{A_c}} = \frac{1}{\sum_{i=1}^{IS-1} \left( \frac{\sum_{j=1}^{IC} L_{ij} / IC}{\sum_{j=1}^{IC} A_{ij}} \right)} \quad (11)$$

which has units of length.

Substituting equations (5), (6), and (10) into equation (3), multiplying by  $I_g$ , and setting  $dQ/dt$  equal to the other terms will obtain:



$$\frac{dQ}{dt} = \frac{-I_g}{2} \left( \frac{1}{A_b^2} - \frac{1}{A_s^2} \right) Q^2 - g I_g (h_b - h_s) - I_g F \quad (12)$$

where  $F$  is defined by equation (10).

If  $M$  inlets connect the bay to the sea, there will be one equation (eq. 12) for each inlet. Let  $Q_m$  be the discharge of the  $m$ th inlet, then the total discharge for all inlets,  $Q_T$ , is the sum of inlet discharges:

$$Q_T = \sum_{m=1}^M Q_m \quad (13)$$

The rate of change of water level in the bay,  $dh_b/dt$ , is related to inlet discharge,  $Q_T$ , plus discharge into the bay from other sources,  $Q_{inflow}$ , by the continuity equation:

$$\frac{dh_b}{dt} = \frac{Q_T}{A_{bay}} + \frac{Q_{inflow}}{A_{bay}} \quad (14)$$

where  $A_{bay}$  is the instantaneous surface area of the bay.

There are several methods available for solving the simultaneous differential equations (eq. 12 for  $M$  inlets, and eq. 14 for a total of  $M+1$  equations). The method selected for this model is a fourth-order Runge-Kutta-Gill technique. Advantages of this method are that it is self-starting, extremely stable, may use a longtime step, has wide application, and converges quickly. The main disadvantage of this technique is that it may cost approximately twice as much as some methods because the Runge-Kutta-Gill technique uses two calculations for each time step to check error bounds and establish the time step (International Business Machine, 1970). On a CDC 6600 computer the total computer cost for computations per inlet for a tidal cycle using the Runge-Kutta-Gill method has been, at most, several dollars (less than \$2 per tide cycle) for most test inlets. However, the ease in use of this method justifies its cost.

The computer program based on this model (INLET) is presented in Appendix D.

### 3. Weighting Functions.

The weighting function,  $W_{ij}$  (the fraction of the total flow that passes through a grid cell at a time step), provides a systematic method of distributing flow throughout an inlet for use in evaluating the bottom stress (see App. A). There are three weighting functions developed in this report.

First, a weighting function to distribute flow between channels at each cross section so that total friction in the section is minimized (option IWT=1 in the computer program INLET). This function may allow some water to move perpendicular to the main axis of the flow net at each cross section, but the flow should be small for a well-drawn net.

Second, a weighting function is developed by assuming that all flow is parallel to the streamlines of the flow net and distributes the discharge in each channel of the inlet to minimize overall friction (option IWT=2 in the computer program INLET). This method is consistent with the equations of motion used in the model derivation and assumes that the grid system permits an accurate representation of the inlet streamline patterns.

In practice both of the minimum friction weighting functions (IWT=1 and IWT=2) produce similar results for many inlets. Comparisons with prototype measurements show that either minimum friction weighting function adequately predicts the flow distribution across inlet cross sections tested. For example, at Brown Cedar Cut, Texas, minimum friction weighting predicted the fraction of flow at any point at a cross section to within several percent of the measured total flow (Fig. 3). At times in the tidal cycle, minimum friction weighting slightly over or under predicts flow in parts of the section, but on the average this difference will have little effect on the final result.

Differences between prototype and minimum friction weighting distributions may occur for several reasons. First, Manning's uniform flow friction relation, as used in this model, may not completely describe friction losses for unsteady inlet flow. For example, Manning's equation may overestimate friction when discharge is increasing because turbulence has not developed to the point that would be reached for steady flow of the same discharge. As the magnitude of discharge decreases, the opposite effect occurs. Turbulence is higher than for the same steady-state discharge, so Manning's equation underestimates actual friction. Changes in bed forms throughout the tide cycle may also change frictional resistance.

Jet formation in flow exiting the inlet throat, not accounted for in these weighting functions, may also cause differences between observed and predicted flow patterns. However, an empirical coefficient may be used to account for losses caused by jets (discussed in App. C).

See Appendix B for a detailed discussion of drawing flow nets based on the minimum friction weighting function (IWT=1).



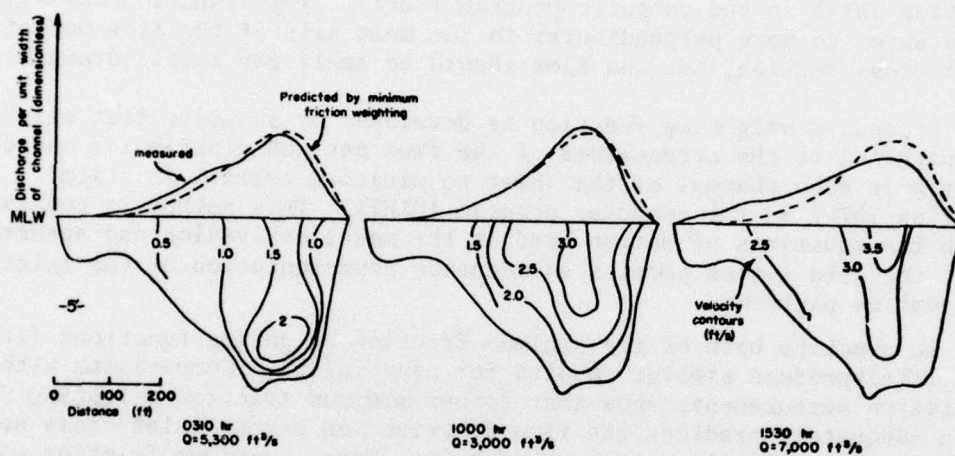


Figure 3. Discharge of Brown Cedar Cut, Texas (after Mason and Sorensen, 1971).

The third weighting function (IWT=3) is the same as that used in lumped parameter models (Huval and Wintergerst, 1977). It assumes that discharge is the same in all grid cells throughout the inlet at any instant in time. This means that the flow in each grid is equal to the total flow for the inlet divided by the number of channels, IC. The weighting function,  $W_{ij}$  is evaluated by:

$$W_{ij} = 1.0/IC \quad (15)$$

Generally, this function will not accurately reproduce prototype conditions because it is almost impossible to visually draw the grid that corresponds to this condition. In addition, the function does not consider changes in flow distribution that occur during the tidal cycle. However, this weighting function is useful in obtaining an upper limit for frictional effects because it overemphasizes friction in most cases. By producing high friction the function tends to give a lower limit for bay level fluctuations, mean inlet velocity, and inlet discharge.

Weighting functions are organized as subroutines in the computer program INLET, so that other weighting functions may easily be added to this numerical model as they are developed.

#### IV. USE OF THE COMPUTER PROGRAM

##### 1. General Conditions for Model Use.

The computer program INLET (App. D) estimates inlet hydraulics by solving equations (12), (13), and (14) with the use of appropriate geometric and hydraulic inputs. This section summarizes the conditions that can be modeled consistent with assumptions discussed in Section III. Details of the application of the computer program are discussed in Appendix D.

The conditions required when using the model are:

- (a) The sea is much larger than the inlets and bay.
- (b) The bay is much larger than the inlets.
- (c) The bay and at least one of the inlets must contain water throughout the water level cycle.
- (d) The forcing seawater level fluctuations must be specified as a function of time. Start the model when the sea level is zero.
- (e) The inflow or outflow from sources other than the inlet must be a specified function of time, a function of bay water level, or constant (the computer model is programmed for a constant inflow).



(f) The bay surface area must be constant, a function of bay water level, or a function of time (the model is now programmed for bay area as a linear function of bay level).

(g) The inlet cross-sectional area at a section is a function of local water level or time (the model is programmed for linear channel side slopes).

(h) The model assumes that the bay level remains horizontal. This means that the forcing wavelength in the bay should be much longer than the bay:

$$T_F \sqrt{gd_{bay}} \gg L_{bay} , \quad (16)$$

where  $L_{bay}$  is the length of the longest axis of the bay,  $d_{bay}$  is the depth of the bay, and  $T_F$  is the forcing wave period.

(i) This model uses a lower limit of the time step,  $\Delta t$ , which is the time required for a shallow-water wave to travel the length of the inlet channel:

$$\Delta t = \frac{L_{in}}{\sqrt{gd_{max}}} \quad (17)$$

where  $L_{in}$  is the length of the inlet, and  $d_{max}$  is the maximum water depth in the inlet. In practice, a much longer time step may be used in some situations; the Runge-Kutta-Gill technique used in INLET will automatically adjust the time step. A suggested upper limit for the time step input to INLET is one-hundredth of the forcing wave period,  $T_F$ .

(j) Recommended techniques for drawing inlet grid flow nets, which are used in the evaluation of bottom friction, are presented in Appendix B.

(k) The weighting function for distributing the flow throughout the inlet must be selected (Sec. II). The minimum friction weighting functions (option IWT=1 or IWT=2) are recommended for most cases.

(l) If the seawater level fluctuation is of constant period and amplitude, generally one to four cycles are required to eliminate transient terms so that an equilibrium response is reached in the inlet and bay. If friction is high, as in many tidal inlets, the first cycle will give a good estimate of water motions and levels. Four cycles are recommended for inlets

with high temporal acceleration, such as inlets on the Great Lakes.

(m) The program INLET is designed so that the sea forcing is sinusoidal or is described by water levels sampled at a uniform rate,  $Q_{inflow}$  is constant,  $A_{bay}$  is a linear function of the bay water level, and the inlet side slope is linearly related to the local water depth. For more general conditions, appropriate programing changes should be made to the computer program. For example, if the surface area of the bay is a nonlinear function of bay water level, the program statement that evaluates bay surface area should be changed to incorporate the given function.

(n) The recommended method of using this model is similar to that for other inlet models. Obtain prototype geometries, sea and bay water levels, and inlet velocities for the system. Using this prototype information, calibrate the model so that the predicted bay levels and inlet velocities accurately reproduce the prototype data. The suggested method of calibration is to vary the Manning's bottom friction factor,  $n$ , or ebb and flood entrance and exit-loss coefficients (App. C) until good agreement is obtained between observed and predicted hydraulics. For short records of field data it is recommended that the model first be calibrated so that predicted inlet velocities or discharges reproduce prototype conditions, because velocities are most sensitive to changes in model parameters. Bay levels are a form of integrated inlet discharge, so levels are less sensitive to variation in model parameters. After the model is calibrated for velocities, check bay level predictions.

If additional prototype data are available, the calibrated model should be run with the additional data to verify that the model produces adequate results.

If no prototype data are available for calibration, use values of  $n$  and loss coefficients previously calibrated on the model for similar inlets. The examples presented later in this section provide estimates of values that apply to various types of inlets. Specific applications of the model are also presented.

A preliminary estimate of  $n$  can be obtained from the linear relation recommended by Masch, Brandes, and Reagan (1977) for grid cells of tidal inlet models. They reported that  $n$  is weakly dependent on water depth by the relation:

$$n = C_1 - C_2 D \quad (18)$$



where  $D$  is the stillwater depth, because the probability of vegetation decreases with depth. For water depths less than 9.1 meters (30 feet) and greater than 1.2 meters (4 feet) they recommend  $C_1 = 0.0377$  and  $C_2 = 0.000667$ . This condition is assumed if a relation for  $n$  is not specified in input data. For depths less than 1.2 meters they recommend  $C_1 = 0.055$  and  $C_2 = 0.005$ .

(o) Note that asymmetry of the inlet and bay geometries throughout the forcing cycle will result in asymmetrical inlet hydraulics. For example, a sinusoidal ocean tide may produce a nonsinusoidal bay tide due to effects such as the change in inlet cross-sectional area with water depth (Keulegan, 1967).

## 2. Examples.

The model was applied to a variety of tidal and nontidal inlets. Five examples were selected to illustrate the range of conditions that can be modeled: Pentwater Inlet, Michigan, to show the response of a simple geometry, nontidal inlet to forcing at different wave periods due to seiching of Lake Michigan; a hypothetical harbor, which illustrates the application of the model to predict tsunami-induced hydraulics at a harbor; Masonboro Inlet, North Carolina, to illustrate the evaluation of tidal inlet hydraulics; Indian River, Delaware, to show how the model can predict the effect of storm surge at a tidal inlet; and Cabin Point Creek, Virginia, to show the effect of adding a second inlet to a one-inlet tidal system.

a. Pentwater Inlet, Michigan. Pentwater Inlet, located on the east coast of Lake Michigan, is an example of a nontidal Great Lakes inlet (Fig. 4). Like many Great Lakes inlets, Pentwater is controlled by parallel jetties and retaining walls, and is periodically dredged to maintain the channel.

(1) Geometry. The channel is 610 meters (2,000 feet) long with a width of 44 meters (145 feet) and a minimum depth of 3.6 meters (12 feet). A survey of the channel at six cross sections with a spacing of 122 meters (400 feet) was used to calculate the cross-sectional flow area along the inlet (Fig. 5). Since the inlet cross-sectional area and depth are approximately uniform, the inlet was modeled using a one-channel flow net. The area of the bay, Pentwater Lake ( $1.68 \times 10^6$  square meters,  $1.812 \times 10^7$  square feet), was measured from Lake Survey Chart 77.

Figure 6 summarizes geometric measurements used in the Pentwater Inlet model.

(2) Forcing Hydraulics. Spectral analysis of water levels in Pentwater Lake indicates that astronomical tides are not important and that Pentwater is being forced by some higher seiching modes of Lake Michigan (Fig. 7). For example, the 5.3-hour peak in the spectra corresponds to the second longitudinal mode of oscillation of Lake Michigan; the 3.5-hour peak is the third mode, etc. (Seelig and Sorensen, 1977).

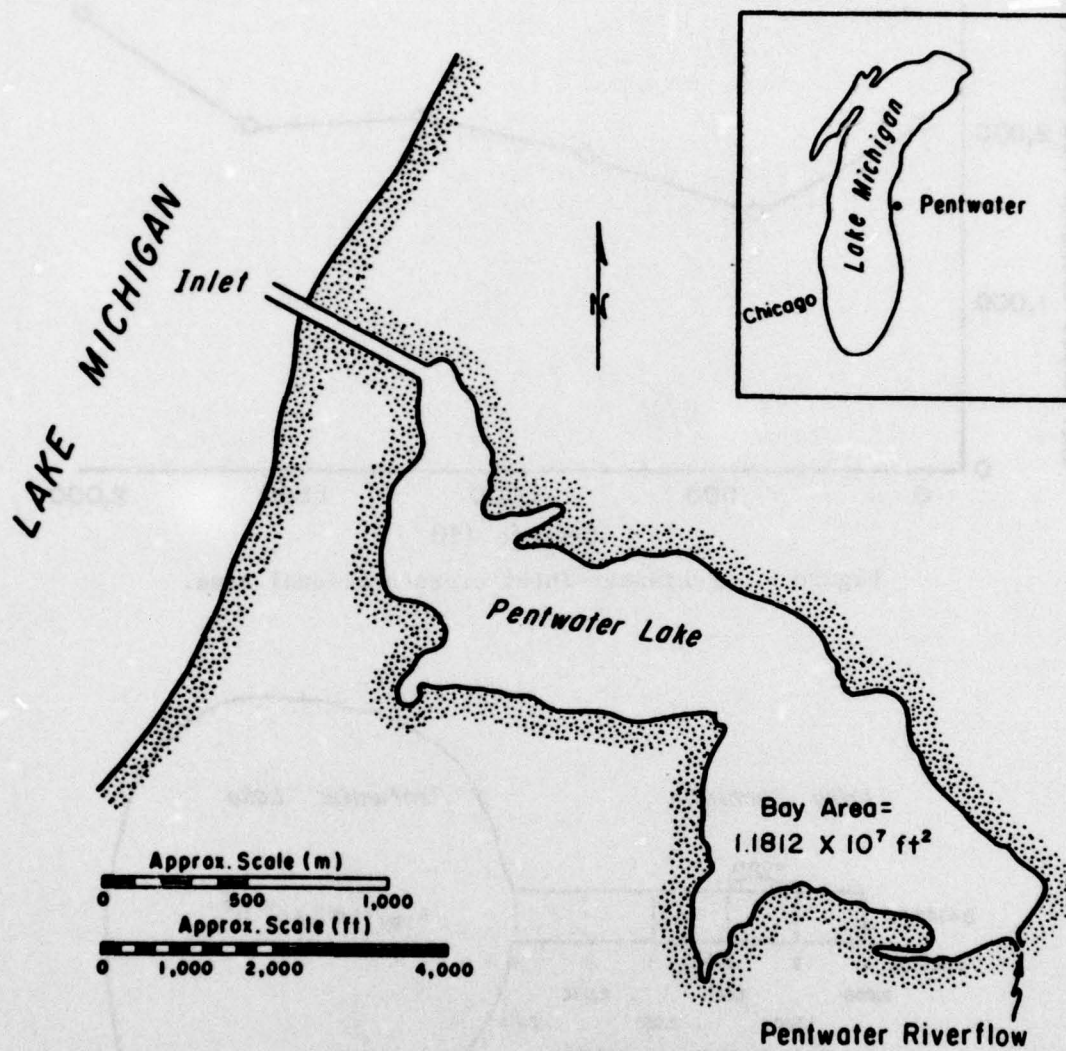


Figure 4. Pentwater, Michigan.



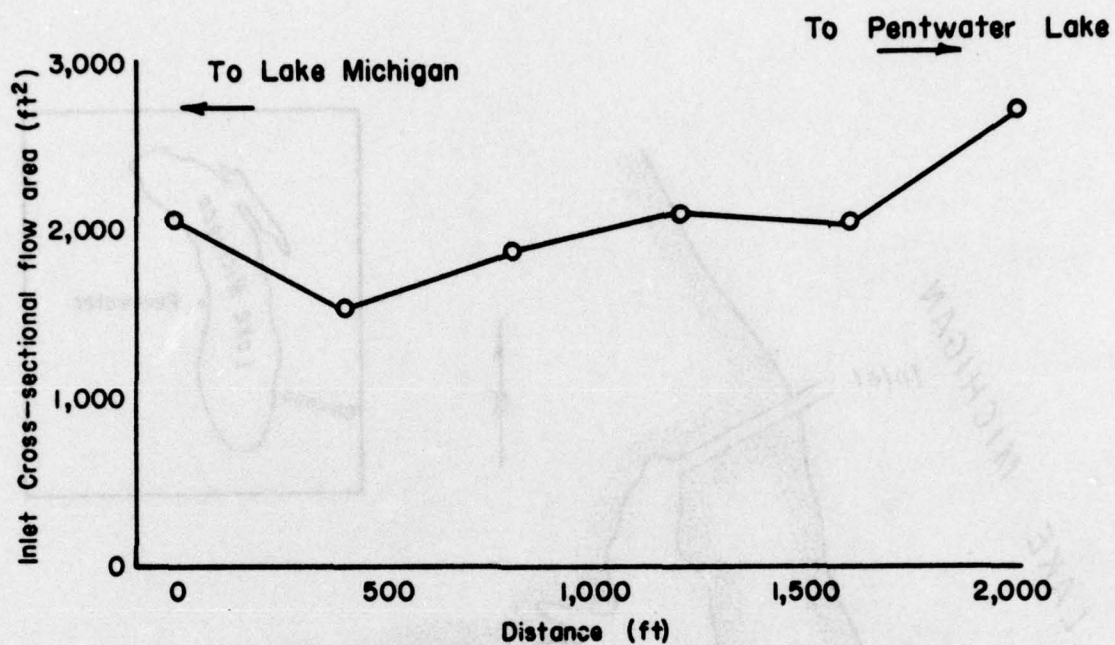


Figure 5. Pentwater Inlet cross-sectional area.

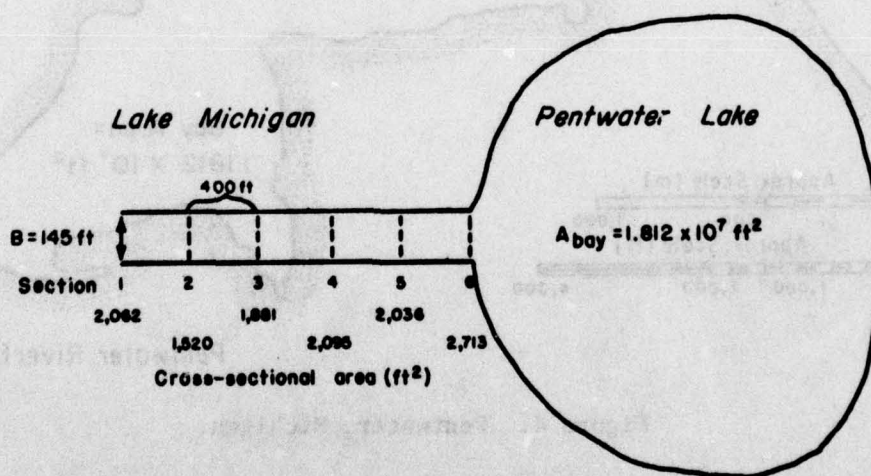


Figure 6. Pentwater geometry used in the numerical model..

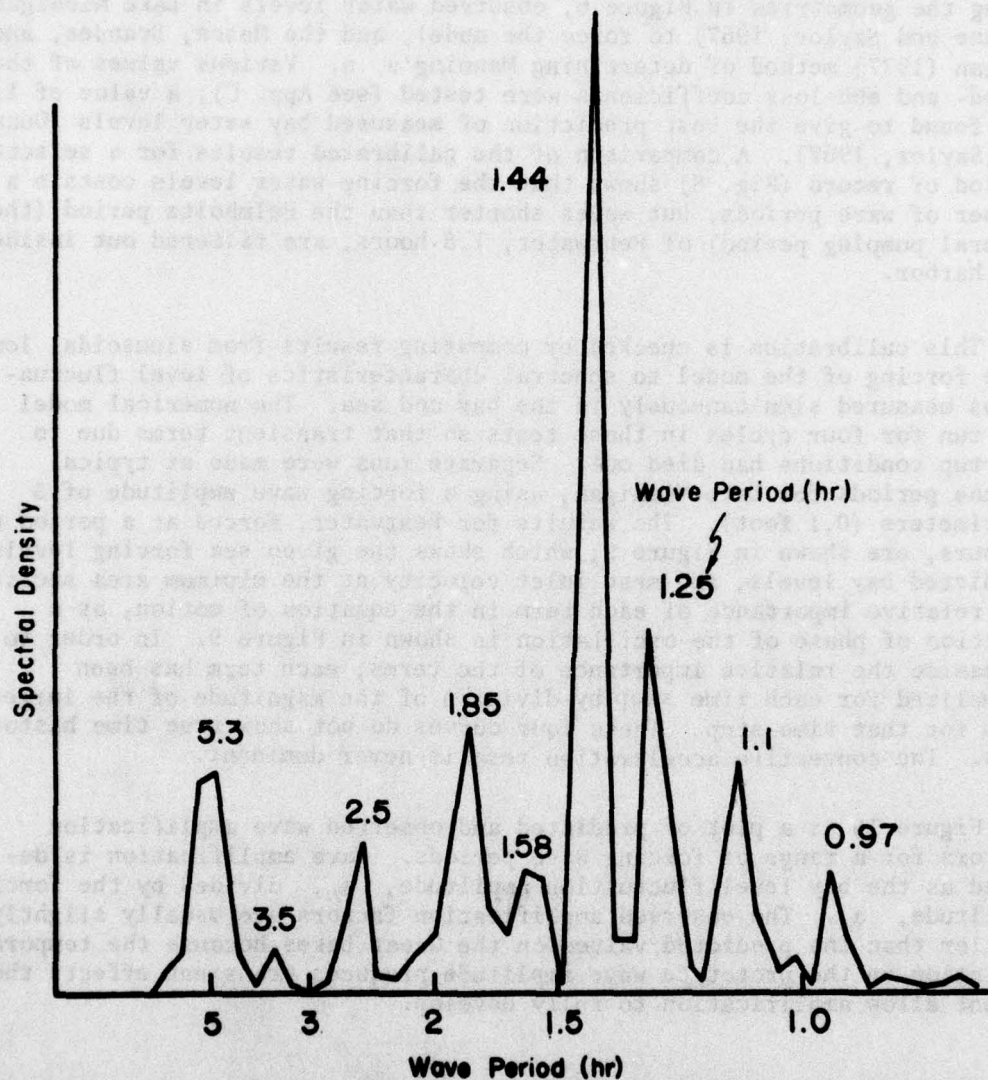


Figure 7. Spectral analysis of Pentwater Lake water levels (3, 4, and 5 November 1974).



(3) Calibration. The Pentwater Inlet model was calibrated by using the geometries in Figure 6, observed water levels in Lake Michigan (Duane and Saylor, 1967) to force the model, and the Masch, Brandes, and Reagan (1977) method of determining Manning's  $n$ . Various values of the flood- and ebb-loss coefficients were tested (see App. C); a value of 1.0 was found to give the best prediction of measured bay water levels (Duane and Saylor, 1967). A comparison of the calibrated results for a selected period of record (Fig. 8) shows that the forcing water levels contain a number of wave periods, but waves shorter than the Helmholtz period (the natural pumping period) of Pentwater, 1.8 hours, are filtered out inside the harbor.

This calibration is checked by comparing results from sinusoidal long wave forcing of the model to spectral characteristics of level fluctuations measured simultaneously in the bay and sea. The numerical model was run for four cycles in these tests so that transient terms due to startup conditions had died out. Separate runs were made at typical seiche periods for Lake Michigan, using a forcing wave amplitude of 3 centimeters (0.1 foot). The results for Pentwater, forced at a period of 2 hours, are shown in Figure 9, which shows the given sea forcing levels, predicted bay levels, and mean inlet velocity at the minimum area section. The relative importance of each term in the equation of motion, as a function of phase of the oscillation is shown in Figure 9. In order to emphasize the relative importance of the terms, each term has been normalized for each time step by division of the magnitude of the largest term for that time step. These four curves do not show true time histories. The convective acceleration term is never dominant.

Figure 10 is a plot of predicted and observed wave amplification factors for a range of forcing wave periods. Wave amplification is defined as the bay level fluctuation amplitude,  $a_b$ , divided by the forcing amplitude,  $a_o$ . The observed amplification factors are usually slightly smaller than the predicted values on the Great Lakes because the temporal variation in the prototype wave amplitude produces transient effects that do not allow amplification to fully develop.

The maximum amplification at Pentwater occurs at a forcing period of 1.8 hours (Fig. 10) and the maximum velocity occurs at a period of 1.4 hours, assuming a constant forcing amplitude of 3 centimeters. The relatively high amplification of the forcing wave, the short Helmholtz period, and the large ratio between surface area of the bay and inlet cross-sectional area ( $10^4$ ) cause relatively large inlet velocities at Pentwater (0.6 meter (2 feet) per second).

b. Tsunami Effects in a Planned Inlet. This model may also be used to predict the effects of a tsunami at some inlets. This example analyzes the response of a hypothetical inlet-harbor system to an assumed tsunami.

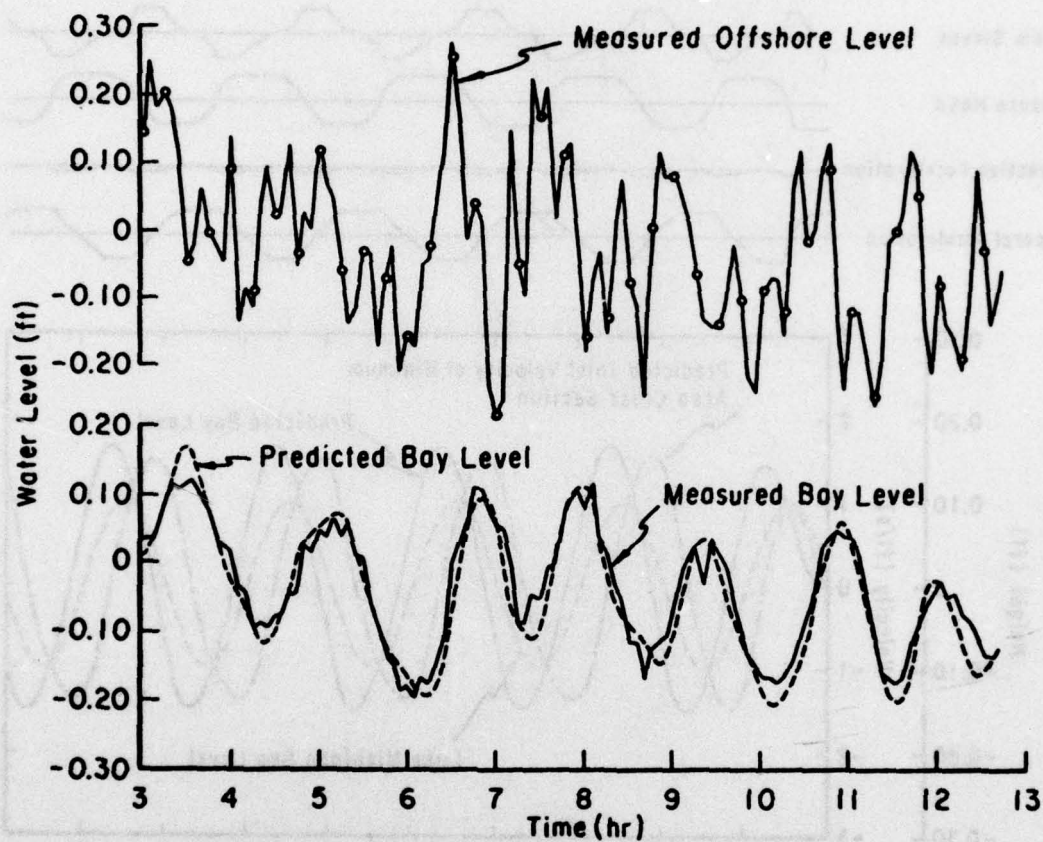


Figure 8. Comparison of results from the Pentwater model application, 17 and 18 August 1967.



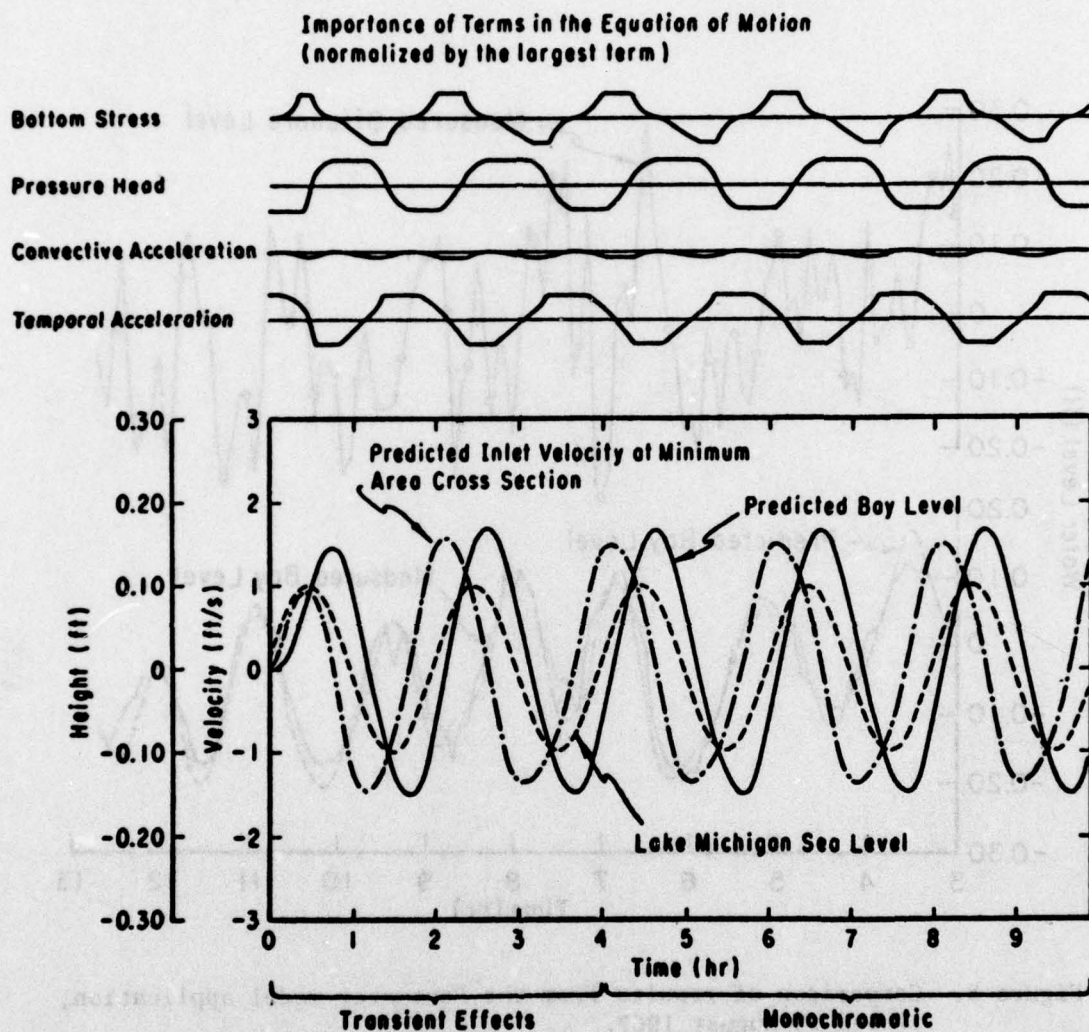


Figure 9. Pentwater model prediction of monochromatic forcing,  $T_F = 2.0$  hours and  $a_0 = 3$  centimeters (0.1 foot).

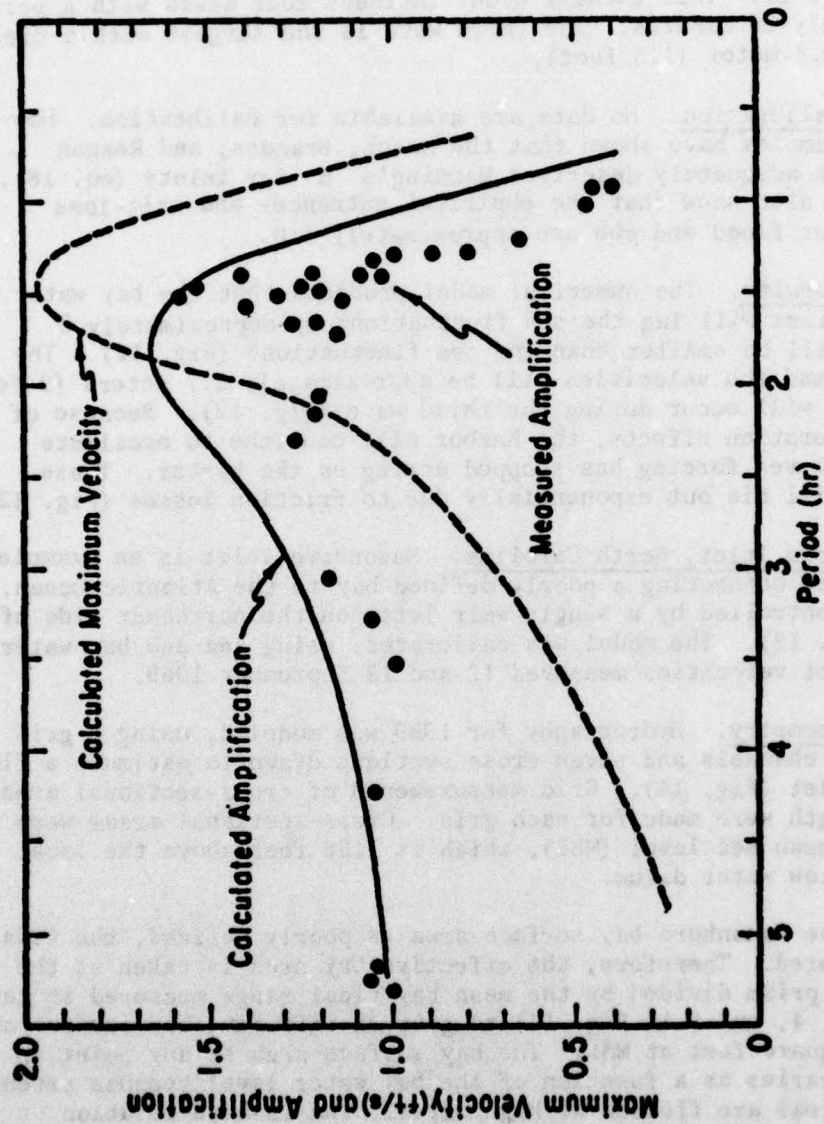


Figure 10. Response to long wave excitation at Pentwater, Michigan (wave amplitude = 0.1 foot).



(1) Geometry. The inlet is 122 meters long, and 24 meters (80 feet) wide with a design depth of 7.3 meters (24 feet). The bay area is  $4.6 \times 10^5$  square meters ( $5 \times 10^6$  square feet).

(2) Forcing. The assumed tsunami seawater level fluctuation is shown in Figure 11. This tsunami event includes four waves with a period of approximately 20 minutes. The third wave is the largest with a crest elevation of 0.4 meter (1.3 feet).

(3) Calibration. No data are available for calibration. However, other examples have shown that the Masch, Brandes, and Reagan (1977) relation adequately describes Manning's  $n$  for inlets (eq. 18). These examples also show that the empirical entrance- and exit-loss coefficients for flood and ebb are approximately 1.0.

(4) Results. The numerical model predicts that the bay water level fluctuations will lag the sea fluctuations by approximately 5 minutes, and will be smaller than the sea fluctuations (Fig. 12). The maximum flood and ebb velocities will be approximately 2.7 meters (9 feet) per second and will occur during the third wave (Fig. 12). Because of temporal acceleration effects, the harbor will continue to oscillate after the sea level forcing has stopped acting on the harbor. These oscillations will die out exponentially due to friction losses (Fig. 12).

c. Masonboro Inlet, North Carolina. Masonboro Inlet is an example of a tidal inlet connecting a poorly defined bay to the Atlantic Ocean. The inlet is controlled by a single weir jetty on the northeast side of the inlet (Fig. 13). The model was calibrated, using sea and bay water levels and inlet velocities measured 12 and 13 September 1969.

(1) Geometry. Hydrography for 1969 was modeled, using a grid system of four channels and seven cross sections drawn to estimate a flow net for the inlet (Fig. 14). Grid measurements of cross-sectional area, width, and length were made for each grid. Cross-sectional areas were referenced to mean sea level (MSL), which is 1.88 feet above the local Beaufort mean low water datum.

Although the Masonboro bay surface area is poorly defined, the tidal prism was measured. Therefore, the effective bay area is taken as the measured tidal prism divided by the mean bay tidal range measured at three gages (gages 3, 4, and 5 in Fig. 14) to give an effective bay surface area of  $1.8 \times 10^6$  square feet at MSL. The bay surface area at any point in time,  $A_{bay}$ , varies as a function of the bay water level because extensive lowland areas are flooded at high water. The assumed relation between bay surface area and bay level is

$$A_{bay} = A_0(1 + \beta h_b) \quad (19)$$

with  $\beta = 0.2$  estimated from hydrographic charts.

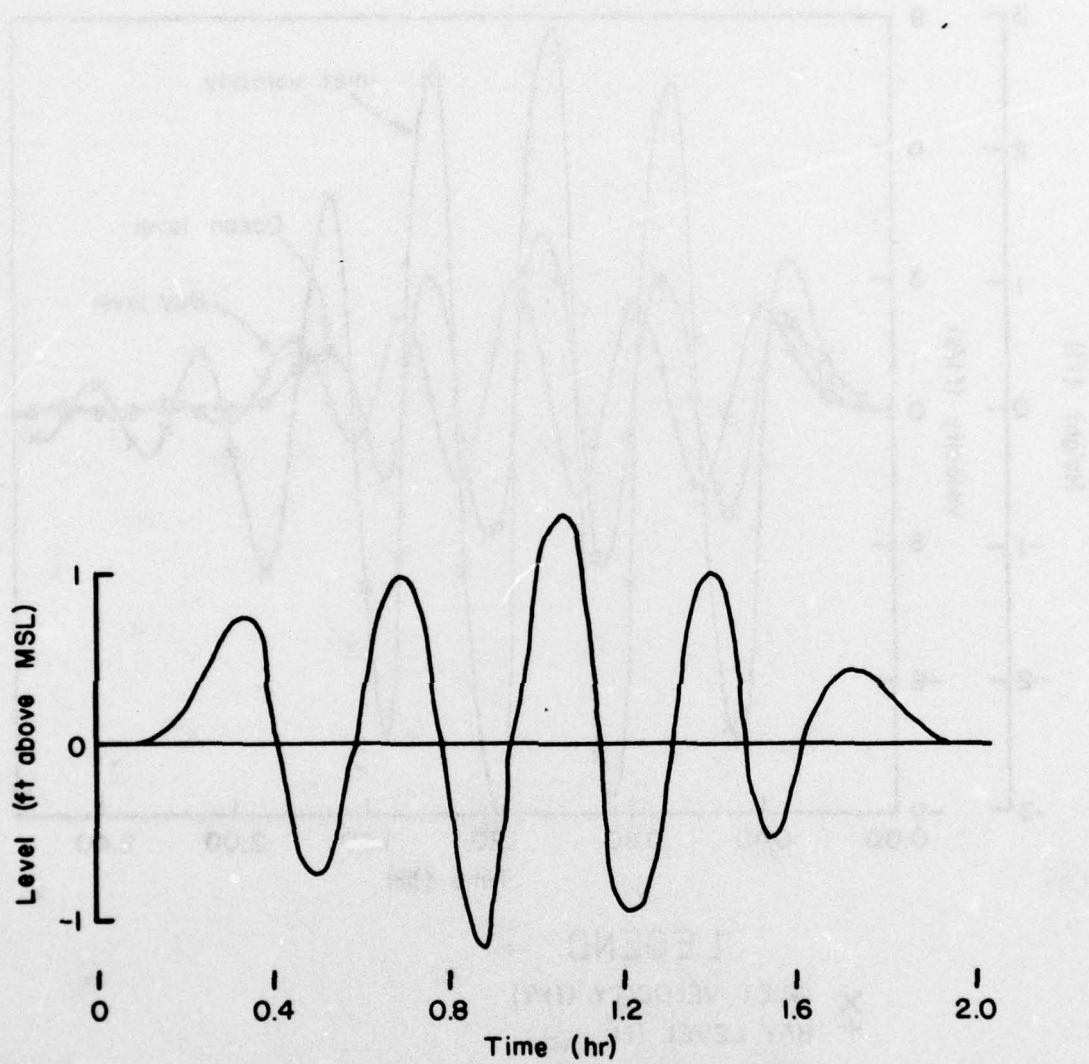
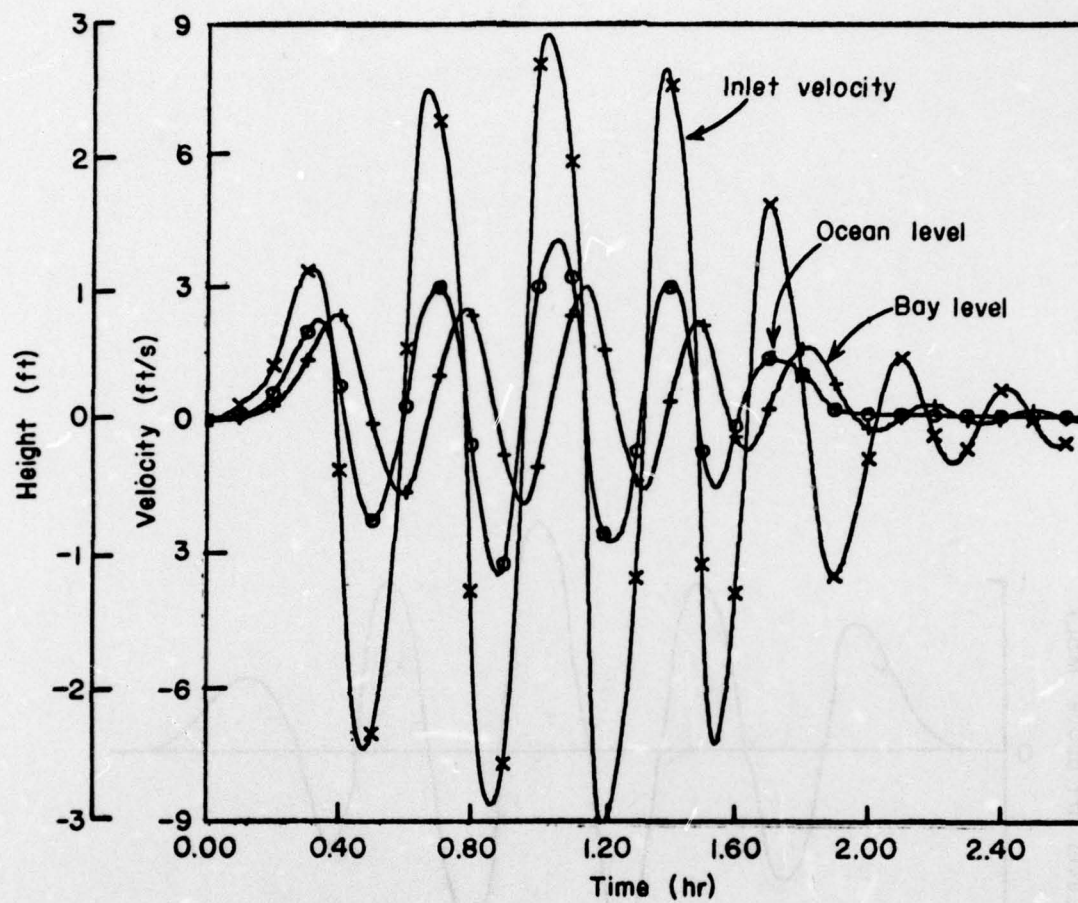


Figure 11. Tsunami water level fluctuations in the sea.





### LEGEND

- x INLET VELOCITY (ft/s)
- + BAY LEVEL (ft)
- OCEAN LEVEL (ft)

Figure 12. Predicted inlet-bay system response to tsunami-generated seawater level fluctuations.



Figure 13. Masonboro Inlet, North Carolina, April 1968.



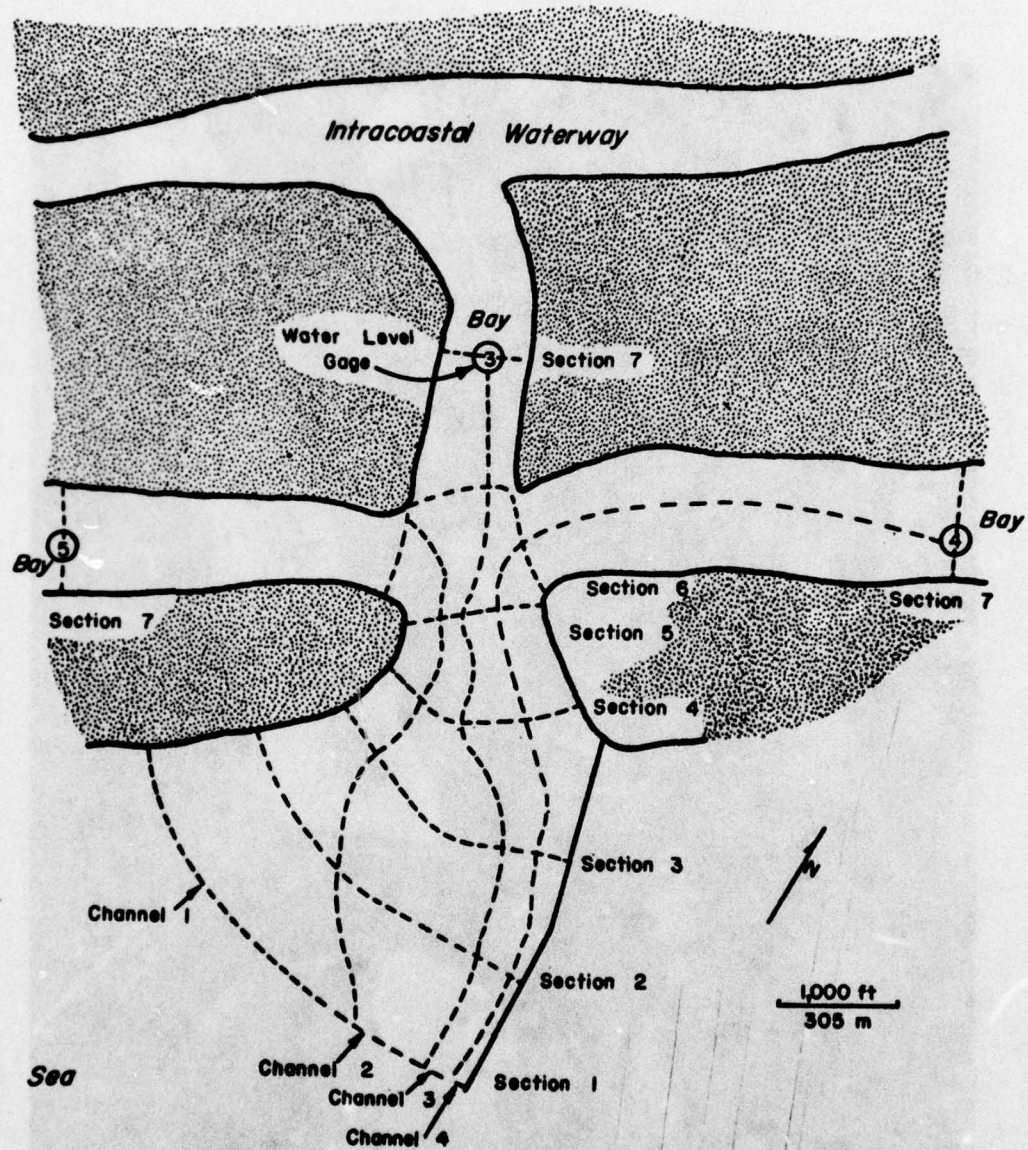


Figure 14. Grid system for Masonboro Inlet, 1969.

The slopes of Masonboro Inlet are estimated to be 0.0133 from the hydrographic survey.

(2) Forcing. Tidewater levels measured outside the inlet at 0.5-hour intervals for a period of 25 hours, 12 and 13 September 1969, were used as the input to the model.

(3) Calibration. The prototype inlet velocities measured 12 September 1969 were used to calibrate the Masonboro model. Values of Manning's  $n$  throughout the inlet grid system are assumed to be a function of the water depth below datum in each grid,  $D_{ij}$ , using the relation recommended by Masch, Brandes, and Reagan (1977):

$$n = 0.0377 - 0.000667 D_{ij} \quad (20)$$

Flow is distributed throughout the inlet so that friction is minimized in each channel (IWT=2).

Figure 15 shows the forcing tide, predicted bay levels, inlet velocity at the throat, and discharge. The plot of the importance of terms in the equation of motion shows that the Masonboro Inlet flow is predominantly controlled by head and frictional effects. Temporal acceleration becomes important only for a short time near slack water.

Comparisons between observed and calibrated inlet velocities show that this is a good model for predicting mean velocity in the throat at Masonboro (Fig. 16). The standard deviation between observed and predicted is 0.62.

This model was also used to estimate flow distribution throughout the inlet using output from the subroutine TABLE in the computer program. Predicted maximum ebb and flood velocities are illustrated in Figures 17 and 18. This calibration shows that highest velocities occur in the deep gorge of the inlet adjacent to the jetty.

d. Indian River Inlet, Delaware. Indian River is an example of a single-jettied inlet which connects an open bay to the Atlantic Ocean (Fig. 19). This example illustrates the effects of a storm surge on water motions in the inlet and bay.

(1) Geometry. The inlet, approximately 1,650 meters (5,400 feet) long, is comparatively straight and uniform throughout its length, so the inlet is modeled using one channel and five cross sections (four grids long). The channel dimensions of 1943 (Keulegan, 1967) were used to model the inlet geometries (Fig. 20). The bay area is taken as  $4.2 \times 10^8$  square feet (Keulegan, 1967) and as a first approximation the effect of "The Ditches" is neglected because flow through "The Ditches" is small compared to the inlet discharge (Keulegan, 1967).



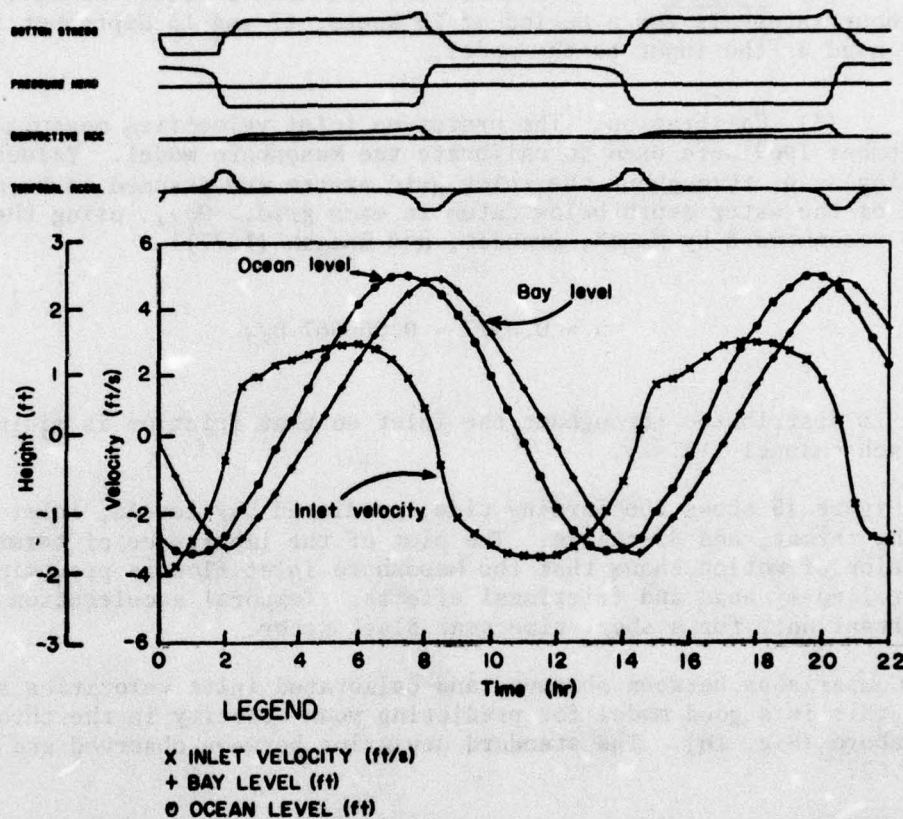


Figure 15. Predicted Masonboro Inlet hydraulics (12 September 1969), CDF = 2.

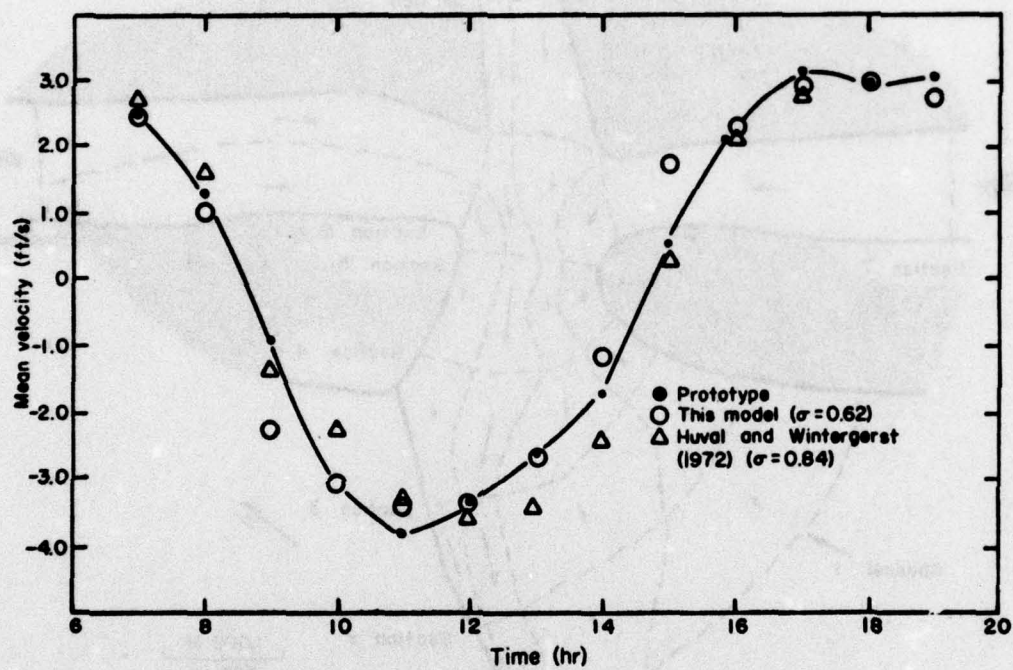


Figure 16. Prototype and predicted mean velocity in the throat of Masonboro Inlet, 12 September 1969.



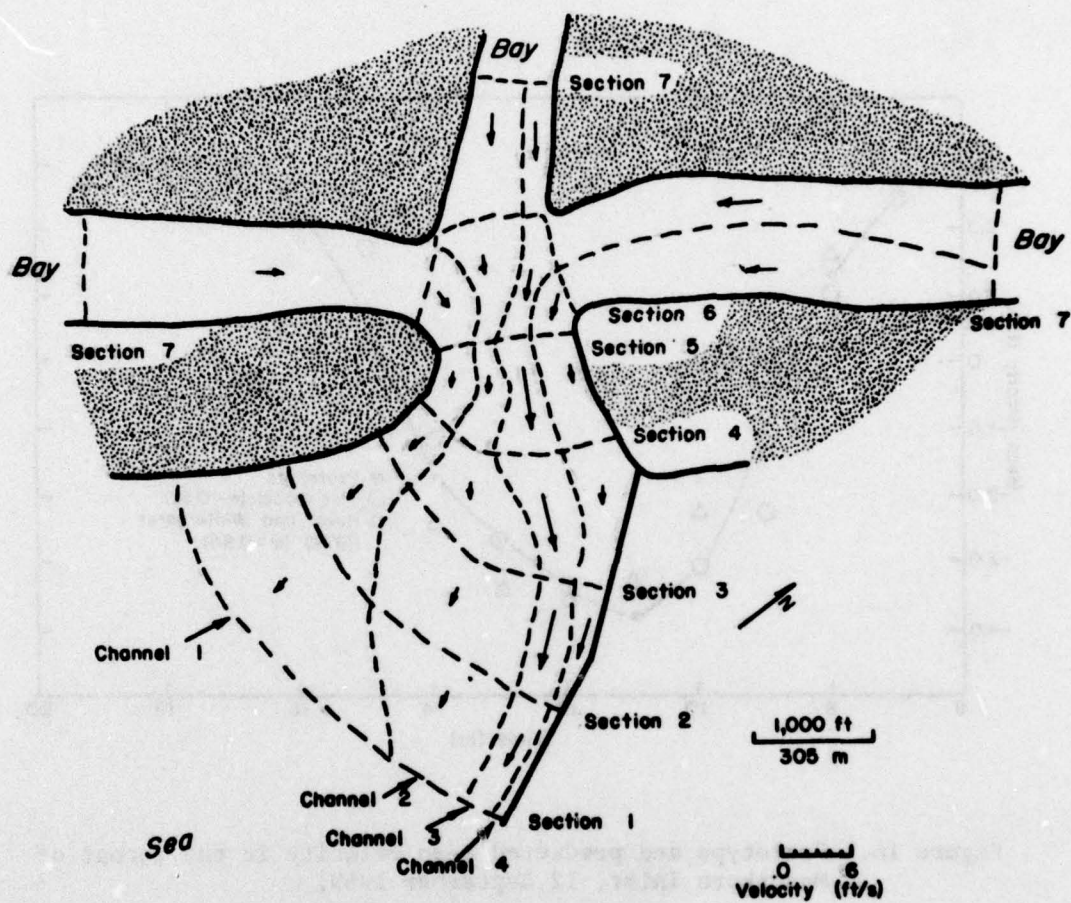


Figure 17. Ebb velocity distribution (minimum friction weighting, 1100 hours, e.s.t.).

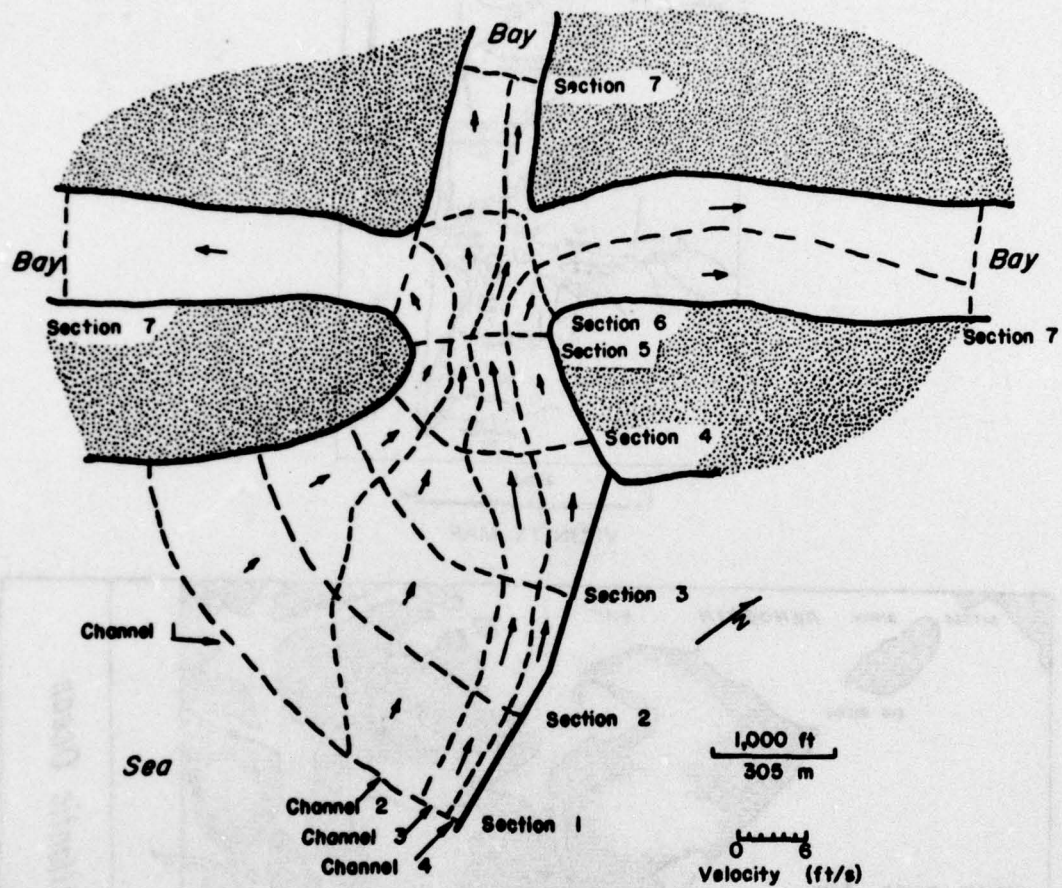
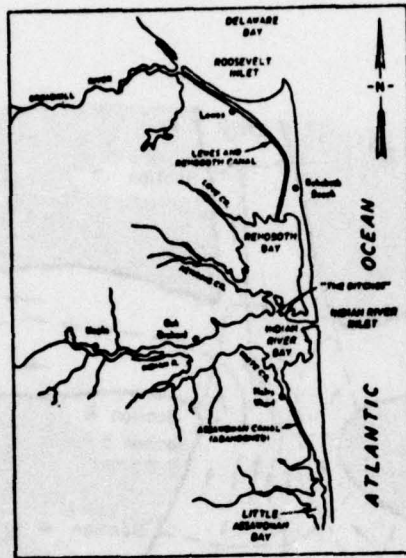
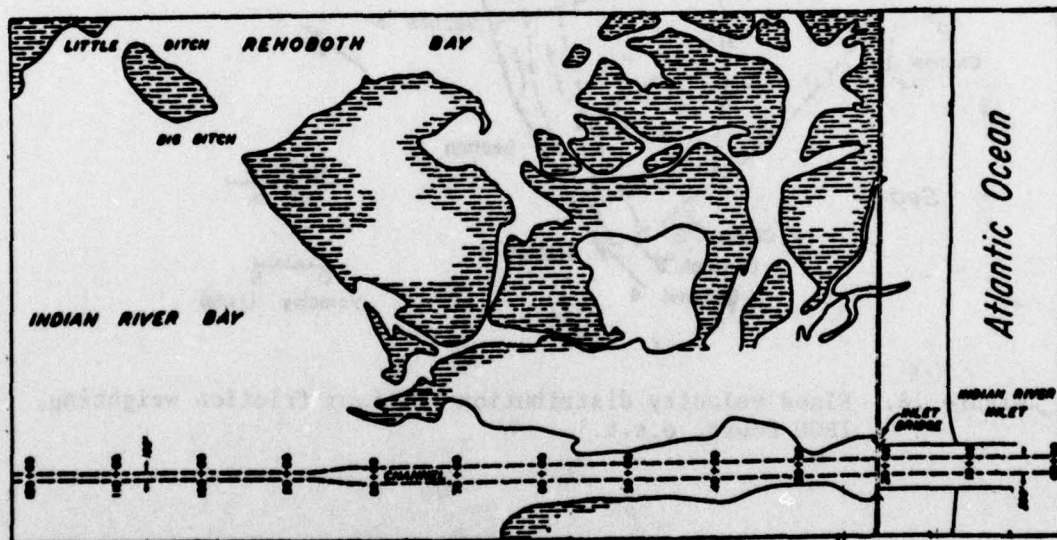


Figure 18. Flood velocity distribution (minimum friction weighting, 1800 hours, e.s.t.).





SCALE  
VICINITY MAP



Scale  
1000 0 1000 2000 ft  
Location Map

Figure 19. Indian River Inlet, Delaware (after Keulegan, 1967).

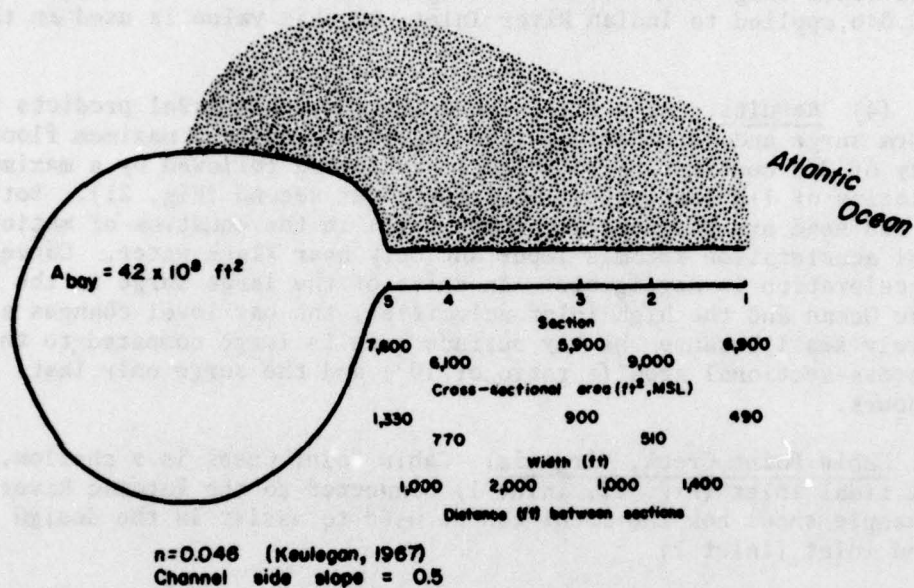


Figure 20. Schematic of Indian River model geometry.



(2) Forcing Hydraulics. The storm surge measured at Atlantic City, New Jersey, 14 and 15 September 1944 is assumed representative of the surge at Indian River (Jelesnianski, 1967). This surge is added to the astronomical tide to obtain a time history of water levels in the Atlantic Ocean at Indian River (Fig. 21).

(3) Calibration. No data are available for calibration of this model to storm surge conditions. Keulegan (1967) found that a Manning's  $n$  of 0.046 applied to Indian River Inlet, so this value is used in the model.

(4) Results. As a first approximation, this model predicts that the storm surge and astronomical tide together produce a maximum flood velocity of 200 centimeters (6.5 feet) per second followed by a maximum ebb velocity of 110 centimeters (3.6 feet) per second (Fig. 21). Bottom stress and head are the most important terms in the equation of motion. Temporal acceleration becomes important only near slack water. Convective acceleration is negligible. In spite of the large surge in the Atlantic Ocean and the high inlet velocities, the bay level changes are relatively small because the bay surface area is large compared to the inlet cross-sectional area (a ratio of  $10^5$ ) and the surge only lasts for a few hours.

e. Cabin Point Creek, Virginia. Cabin Point Creek is a shallow, natural tidal inlet (Fig. 22, inlet 1) connected to the Potomac River. This example shows how the model can be used to assist in the design of a second inlet (inlet 2).

(1) Geometry. The area of the bay, measured from hydrographic charts, is  $3.3 \times 10^5$  square meters ( $3.5 \times 10^6$  square feet) and the length of the natural inlet is 580 meters (1,900 feet). Only one cross section, located one-third of the length of the inlet from the sea, was measured for the inlet in May 1976 (Seelig, 1976). The maximum water depth was 0.6 meter (2.1 feet) with an inlet width of 15 meters (50 feet). This inlet was modeled, using a three-channel flow net. The bay surface variation parameter,  $\beta$ , was determined from hydrographic charts as 0.2.

(2) Forcing. Water level measurements in the Potomac River for an 18-hour interval on 24 and 25 May 1976 were used to force the model.

(3) Calibration. The computer program model of Cabin Point was calibrated by comparing observed and predicted bay levels. Masch, Brandes, and Reagan's (1977) recommended relation between  $n$  and water depth for depths less than 1.2 meters and ebb- and flood-loss coefficients of 1.0 to adequately model bay levels (Fig. 23).

(4) Prediction. The calibrated model was used to predict the hydraulics of the system if a second inlet (hypothetical), 91 meters (300 feet) long, 1.2 meters deep, and 15 meters wide, were cut at location 2 shown in Figure 22. The model predicts that the bay tide range would increase by a factor of four (from 0.36 to 1.49 feet, 0.1 to

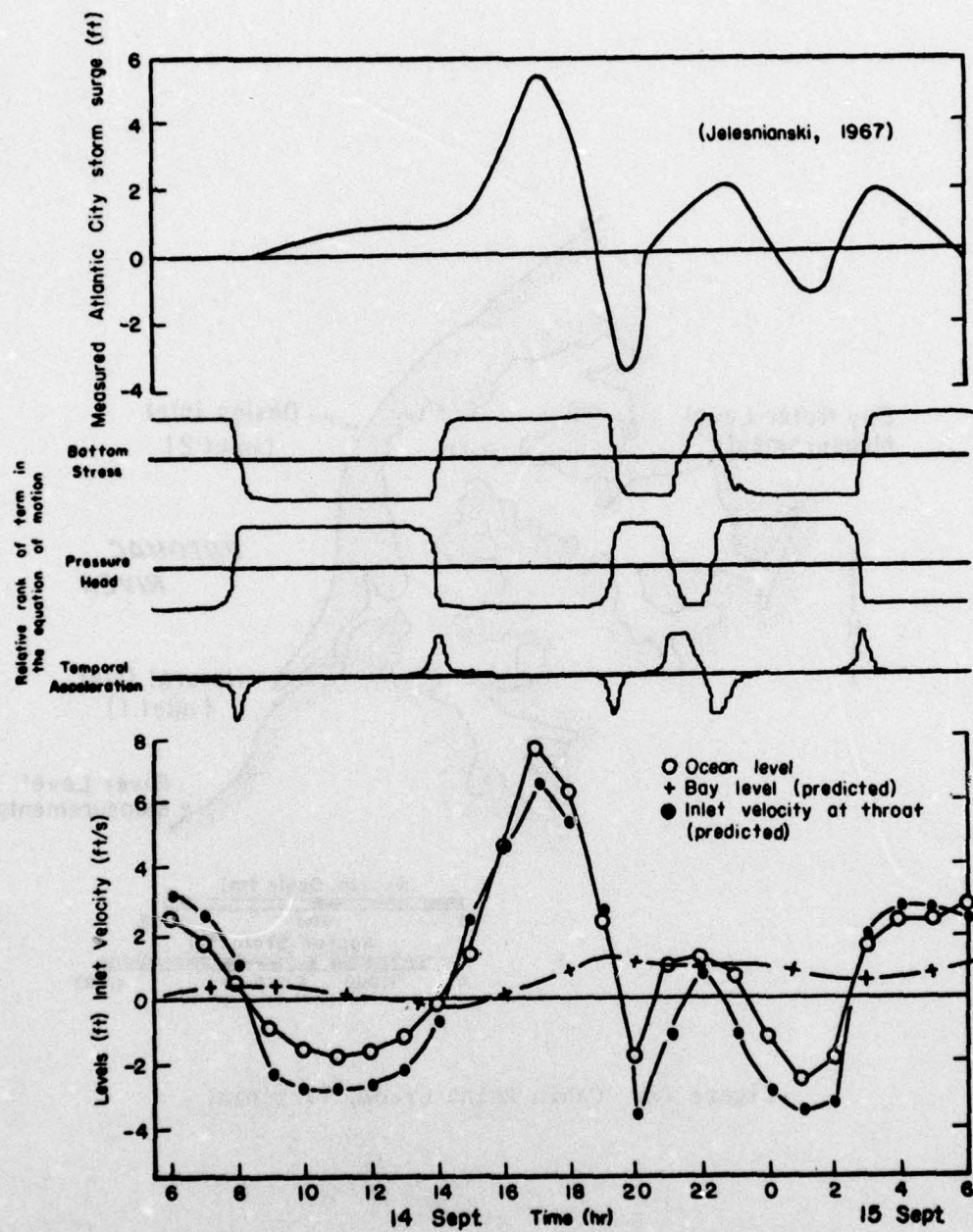


Figure 21. Indian River Inlet channel velocities and bay levels.



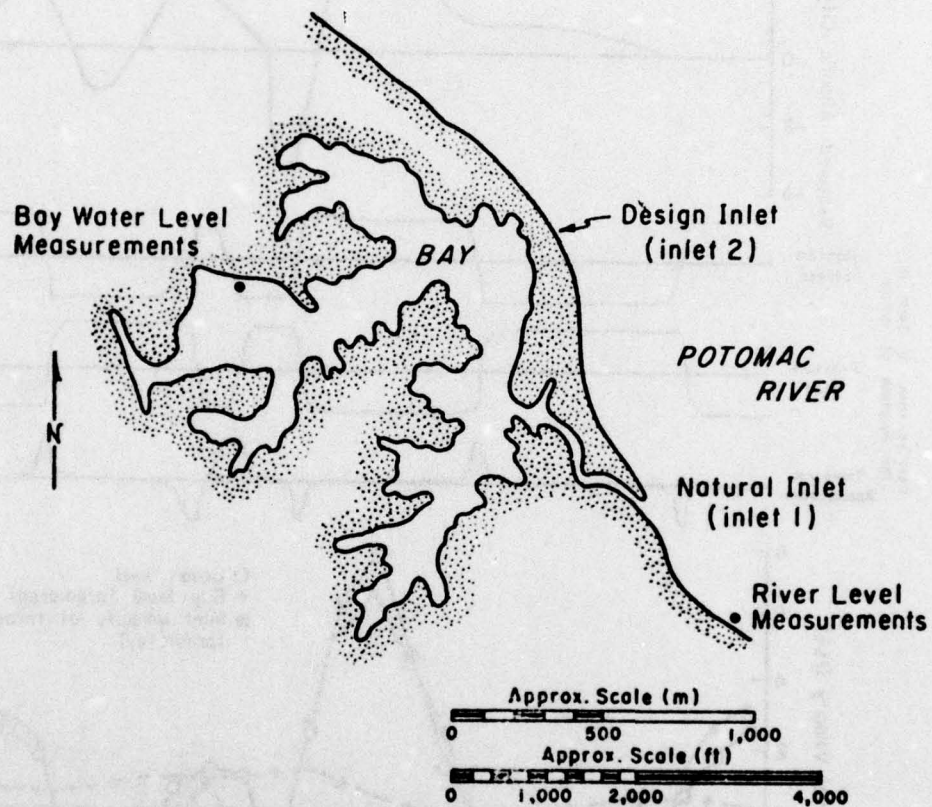


Figure 22. Cabin Point Creek, Virginia.

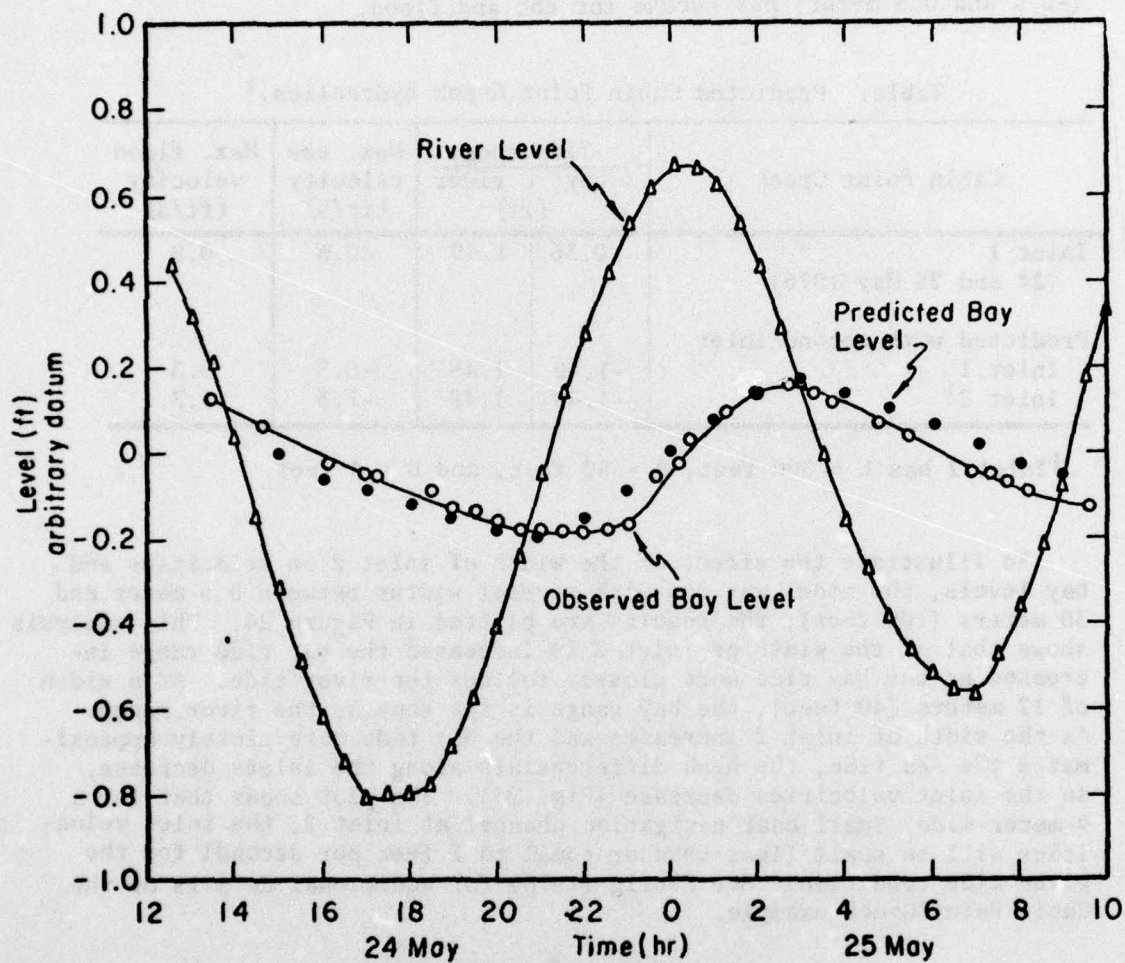


Figure 23. Cabin Point Creek sea and bay levels.



0.45 meter) and the velocities in inlet 1 would decrease (Table 1). The maximum water velocities in inlet 2 would be -1.3 and 1.7 feet (-0.4 and 0.5 meter) per second for ebb and flood.

Table. Predicted Cabin Point Creek hydraulics.<sup>1</sup>

Cabin Point Creek	Tide range		Max. ebb velocity (ft/s)	Max. flood velocity (ft/s)
	bay	river (ft)		
Inlet 1 (24 and 25 May 1976)	0.36	1.49	-0.6	0.9
Predicted with second inlet				
Inlet 1	-1.49	1.49	-0.3	0.3
Inlet 2 <sup>1</sup>	-1.49	1.49	-1.3	1.7

<sup>1</sup>Inlet 2 has L = 300 feet, B = 50 feet, and D = 4 feet.

To illustrate the effect of the width of inlet 2 on velocities and bay levels, the model was run with several widths between 0.6 meter and 30 meters (100 feet); the results are plotted in Figure 24. This analysis shows that as the width of inlet 2 is increased the bay tide range increases as the bay tide more closely follows the river tide. At a width of 12 meters (40 feet), the bay range is the same as the river range. As the width of inlet 2 increases and the bay tide more closely approximates the sea tide, the head differentials along the inlets decrease, so the inlet velocities decrease (Fig. 24). The plot shows that for a 9-meter-wide, small boat navigation channel at inlet 2, the inlet velocities will be small (less than or equal to 2 feet per second) for the given tide condition. See Seelig (1976) for additional details of the Cabin Point Creek example.

## V. SUMMARY AND CONCLUSIONS

A numerical model based on an area averaged momentum equation for the inlet, and a continuity equation for the bay has been shown to give good predictions of bay levels and mean inlet velocities for a variety of inlets and forcing conditions. The model is designed for cases where the bay water level fluctuates uniformly throughout the bay and the volume of water stored in the inlet between high and low water is negligible compared to the prism of water that moves through the inlet.

The ease of use and low cost of this model make it ideal for obtaining a first estimate of inlet hydraulics for inlets forced by the astronomical tide, storm surge, lake seiching, and tsunamis. The computer program INLET (App. D), based on the model, can be used for hydraulic calculations when one or more inlets connect a bay to a sea.

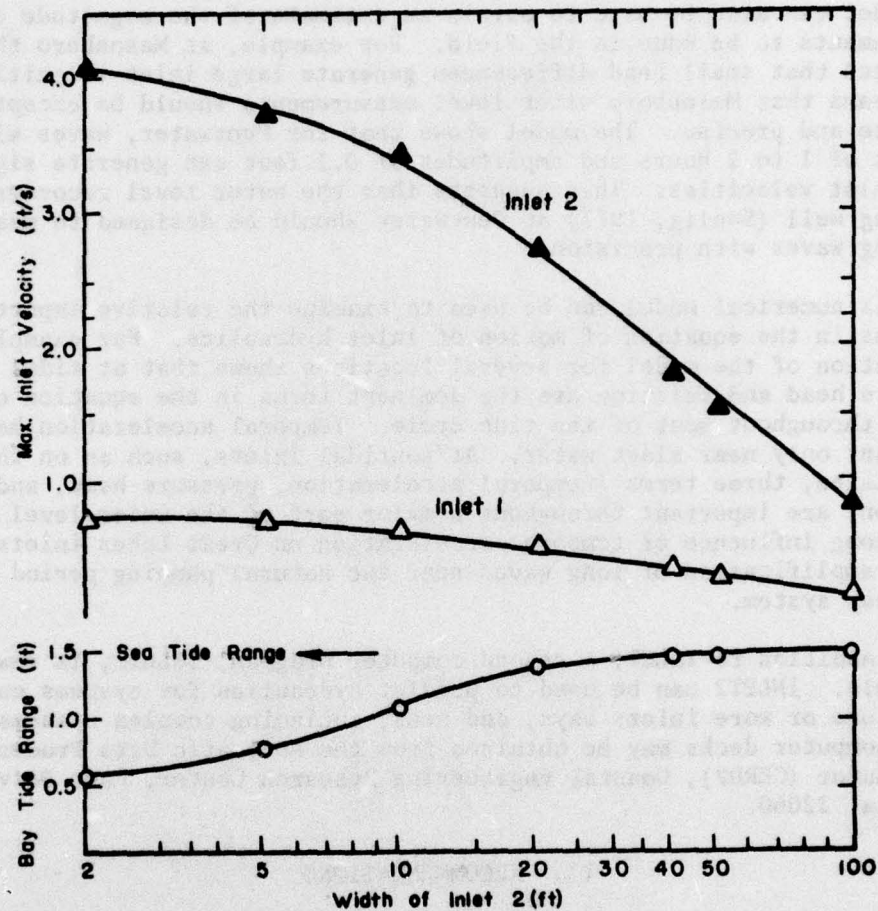


Figure 24. Effect of the width of inlet 2 on inlet and bay hydraulics, Cabin Point Creek, Virginia.



There are several ways that this program can be used to aid the design engineer and researcher. For example, when designing an inlet system many physical parameters can be easily varied in the computer program to determine the influence of each factor on inlet and bay hydraulics. The model can also be used to obtain an estimate of the magnitude of measurements to be made in the field. For example, at Masonboro the model indicates that small head differences generate large inlet velocities. This means that Masonboro water level measurements should be exceptionally accurate and precise. The model shows that for Pentwater, waves with periods of 1 to 2 hours and amplitudes of 0.1 foot can generate significant inlet velocities. This suggests that the water level recorder and stilling well (Seelig, 1977) at Pentwater should be designed to measure the long waves with precision.

This numerical model can be used to examine the relative importance of terms in the equation of motion of inlet hydraulics. For example, application of the model for several locations shows that at tidal inlets, pressure head and friction are the dominant terms in the equation of motion throughout most of the tide cycle. Temporal acceleration becomes important only near slack water. At nontidal inlets, such as on the Great Lakes, three terms (temporal acceleration, pressure head, and friction) are important throughout a major part of the water level cycle. The strong influence of temporal acceleration on Great Lakes inlets causes amplification of long waves near the natural pumping period of the inlet-bay system.

In addition to INLET, a second computer program, INLET2, is now available. INLET2 can be used to predict hydraulics for systems consisting of one or more inlets bays, and seas, including complex systems. These computer decks may be obtained from the Automatic Data Processing Coordinator (CERDP), Coastal Engineering Research Center, Fort Belvoir, Virginia 22060.

## VI. RECOMMENDATIONS

There are several changes recommended for the computer programs INLET and INLET2 to increase the generality of the model. Storage of water in inlets, and radiation and wind stresses may be added to the equations of motion for the inlet. Other weighting functions for distributing flow, for example a weighting function which includes the jet action of water exiting the inlet, may be added to the program. Higher order relations to evaluate bay surface area, inlet cross-sectional area, and inflow into the bay from sources other than the inlet can also be added to the computer program. A bottom-friction relation designed for unsteady flow may slightly increase the quality of the results.

# LITERATURE CITED

- BROWN, E.I., "Inlets on Sandy Coasts," *Proceedings of the American Society of Civil Engineers*, Vol. 54, Part 1, Feb. 1928, pp. 505-553.
- DUANE, D.B., and SAYLOR, J.H., Water Level Records, National Oceanic and Atmospheric Administration, Lake Survey Center, unpublished, 1967.
- HARRIS, D.L., and BODINE, B.R., "Comparison of Numerical and Physical Models, Masonboro Inlet, North Carolina," GITI Report 6, U.S. Army, Corps of Engineers, Coastal Engineering Research Center, Fort Belvoir, Va., and U.S. Army Engineer Waterways Experiment Station, Vicksburg, Miss., June 1977.
- HUVAL, C.J., and WINTERGERST, G.L., "Simplified Numerical (Lumped Parameter) Simulation," App. 4, GITI Report 6, *Comparison of Numerical and Physical Models, Masonboro Inlet, North Carolina*, U.S. Army, Corps of Engineers, Coastal Engineering Research Center, Fort Belvoir, Va., and U.S. Army Engineer Waterways Experiment Station, Vicksburg, Miss., June 1977.
- INTERNATIONAL BUSINESS MACHINES CORPORATION, "System/360 Scientific Subroutine Package, Version II Programmer's Manual," White Plains, N.Y., 1970.
- JELESNIANSKI, C.P., "Numerical Computations of Storm Surges with Bottom Stress," *Monthly Weather Review*, Vol. 95, No. 11, Nov. 1967.
- KEULEGAN, G.H., "Tidal Flow in Entrances Water-Level Fluctuations of Basins in Communication with Seas," T.B. No. 14, Committee on Tidal Hydraulics, U.S. Army, Corps of Engineers, Vicksburg, Miss., July 1967.
- KING, D.B., "The Dynamics of Inlets and Bays," T.R. No. 22, Coastal and Oceanographic Engineering Laboratory, University of Florida, Gainesville, Fla., Mar. 1974.
- MASCH, F.D., BRANDES, R.J., and REAGAN, J.D., "Numerical Simulation of Hydrodynamics (WRE)," App. 2, GITI Report 6, *Comparison of Numerical and Physical Models, Masonboro Inlet, North Carolina*, U.S. Army, Corps of Engineers, Coastal Engineering Research Center, Fort Belvoir, Va., and U.S. Army Engineer Waterways Experiment Station, Vicksburg, Miss., June 1977.
- MASON, C., and SORENSEN, R., "Properties and Stability of a Texas Barrier Beach Inlet," Sea Grant Report TAMU-SG-71-217, Texas A&M University, College Station, Tex., 1971.
- O'BRIEN, M.P., and CLARK, R.R., "Hydraulic Constants of Tidal Entrances," *Proceedings of the 14th Coastal Engineering Conference*, 1974, pp. 1546-1565.



SEELIG, W.N., "Environmental Effects of Inlet Improvement at Cabin Point, Virginia," U.S. Army, Corps of Engineers, Coastal Engineering Research Center, Fort Belvoir, Va., unpublished, July 1976.

SEELIG, W.N., "Stilling Well Design for Accurate Water Level Measurement," TP 77-2, U.S. Army, Corps of Engineers, Coastal Engineering Research Center, Fort Belvoir, Va., Jan. 1977.

SEELIG, W.N., and SORENSEN, R.M., "Hydraulics of Great Lakes Inlets," TP 77-8, U.S. Army, Corps of Engineers, Coastal Engineering Research Center, Fort Belvoir, Va., July 1977.

## APPENDIX A

### WEIGHTING FUNCTIONS TO MINIMIZE FRICTION

Let the total bottom stress in an inlet cross section be described by:

$$F_x = \sum_{i=1}^{IC} \frac{1}{C_i} W_i^2 \quad (A-1)$$

where  $C_i$  is a parameter that can be determined using results from the previous time step:

$$C_i = \frac{A_i^2 (D_i)^{1/3}}{n_i^2 Q^2 B_i L_i} \quad (A-2)$$

where  $W_i$  is an unknown weighting function and  $Q$  is the inlet discharge.  $C_i$  can be evaluated for each grid (as in the computer subroutine WT1), or for each channel (WT2). In many cases, it is reasonable to assume that flow in an inlet is distributed so that total friction is minimized (i.e., flow will follow the path of least resistance). In that case, find  $W_i$  to minimize  $F_x$ . To find  $W_i$  set up a new function,  $F_N$ , which includes a Lagrangian multiplier,  $\lambda$ :

$$F_N = \sum_{i=1}^{IC} \frac{W_i^2}{C_i} + \lambda \left( \sum_{i=1}^{IC} W_i - 1 \right) \quad (A-3)$$

Then differentiate with respect to each  $W_i$  to obtain a series of equations:

$$\frac{\partial F_N}{\partial W_i} = \frac{2W_i}{C_i} + \lambda ; i=1, IC . \quad (A-4)$$

Also differentiate equation (A-3) with respect to  $\lambda$  to obtain:



$$\frac{\partial F_N}{\partial \lambda} = \sum_{i=1}^{IC} w_i - 1. \quad (A-5)$$

Setting equations (A-4) and (A-5) to zero will give the values of  $w_i$  to obtain the minimum  $F_x$ , or

$$w_i = \frac{-C_i \lambda}{2} ; \quad i=1, IC. \quad (A-6)$$

Substituting equation (A-6) into equation (A-5) for  $w_i$ :

$$\sum_{i=1}^{IC} \left( \frac{-C_i \lambda}{2} \right) - 1 = 0 \quad (A-7)$$

and solving for  $\lambda$ :

$$\lambda = \frac{2}{\sum_{i=1}^{IC} (-C_i)} \quad (A-8)$$

Then substitute  $\lambda$  into equation (A-6) to obtain the weighting function to minimize friction:

$$w_i = \frac{C_i}{\left( \sum_{i=1}^{IC} C_i \right)} \quad (A-9)$$

where  $C_i$  is given by equation (A-2).

This function to determine the weighting for minimum friction can be applied in several ways. If flow is allowed to cross channels, the minimum friction weighting can be applied at each cross section of grids (WT1). This will give the lowest possible friction in the model. If flow is not allowed to cross channels, then the routine can be applied to distribute flow in each channel to minimize friction (WT2). This will give more frictional resistance than in WT1.

## APPENDIX B

### CONSTRUCTING FLOW NETS

A flow net for an inlet is a series of subchannels and cross sections that divide the inlet into a set of grid cells. The primary purpose of the flow net is to evaluate bottom friction throughout the inlet at each time step. The process of constructing the flow net is subjective. The subjectivity can be reduced by following the procedures described here. Friction in each cell is determined by taking the mean geometric and hydraulic conditions throughout each cell and applying them at the centroid of the cell through the use of Manning's bottom-friction relationship. Total inlet friction is the sum of friction in all the cells. The flow net should be drawn so that flow parallels the subchannels and is perpendicular to the cross sections. In prototype inlets, the actual flow net system changes with time and these changes are partially accounted for in the model by reevaluating the weighting function ( $IWT=1$  and  $IWT=2$ ) at each time step.

#### 1. Drawing Flow Nets.

One method of drawing a flow net for an inlet is by assuming that flow is distributed at each cross section so that friction is minimized ( $IWT=1$  in the computer program; see App. A). The first procedure for applying this method is to (a) draw a number of cross sections which are approximately perpendicular to flow for the inlet, (b) apply the minimum friction weighting function to each section to determine channel locations so that flow through each channel will be equal, and (c) check to see that the estimated cross sections are perpendicular to the channels drawn in (b). If not, repeat steps (a) and (b).

A hypothetical inlet is used to illustrate the technique for drawing flow nets (Fig. B-1). This idealized inlet has a maximum depth of 5.5 meters (18 feet) below MSL in the throat, is 366 meters (1,200 feet) wide at the minimum width, and is symmetric about the longitudinal axis of the inlet. A deep gorge runs through the centerline of the inlet.

The first step in drawing a flow net grid system is to draw a number of inlet cross sections which are approximately perpendicular to flow in the inlet. These estimated cross sections are drawn, keeping in mind that in the throat and near the centerline of the inlet most flow will be parallel to the main axis of the inlet. Beyond the inlet, the flow will move in more of a radial direction where the inlet has the effect of a source. Figure B-2 illustrates 15 cross sections estimated for the idealized inlet. These sections are placed closer together in the throat of the inlet because this is a high friction region.

The second step is to determine water depths at points along each cross section, and based on this information, determine channel locations so that flow is distributed to minimize the head loss due to





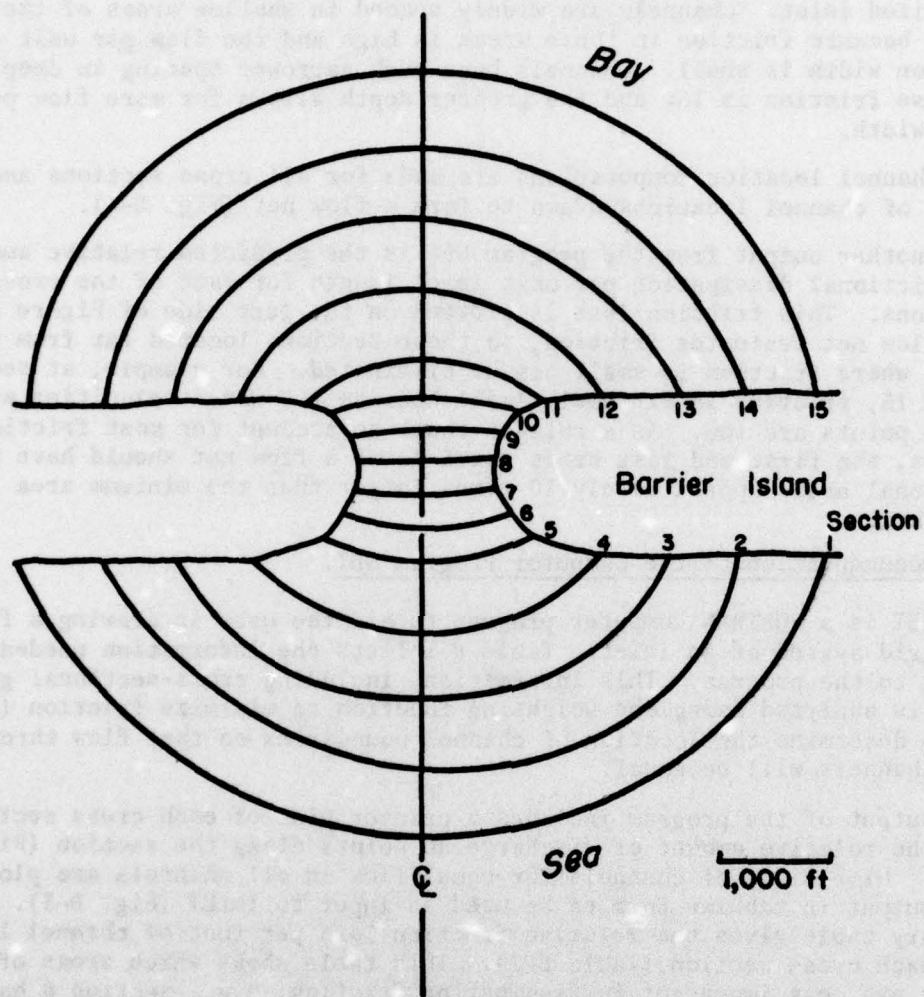


Figure B-2. Selected idealized inlet cross sections.



friction. This task is aided by the FORTRAN computer program, NET, discussed later in this appendix. Figure B-3 shows the channel locations estimated so that flow is equal in all channels for section 12 of the idealized inlet. Channels are widely spaced in shallow areas of the inlet because friction in these areas is high and the flow per unit cross-section width is small. Channels have much narrower spacing in deep areas because friction is low and the greater depth allows for more flow per unit width.

Channel location computations are made for all cross sections and the locus of channel locations drawn to form a flow net (Fig. B-4).

Another output from the program NET is the predicted relative amount of frictional dissipation per unit inlet length for each of the cross sections. This friction loss is plotted on the left side of Figure B-4. The flow net evaluates friction, so those sections located far from the inlet where friction is small can be eliminated. For example, at sections 1 and 15, friction is extremely small because the water velocities at these points are low. As a rule-of-thumb to account for most friction losses, the first and last cross sections of a flow net should have cross-sectional areas approximately 10 times larger than the minimum area in the throat.

## 2. Documentation of the Computer Program NET.

NET is a FORTRAN computer program to aid the user in drawing a flow net grid system of an inlet. Table B-1 lists the information needed for input to the program. This information, including cross-sectional geometry, is analyzed using the weighting function to minimize friction (App. A) to determine the location of channel boundaries so that flow through all channels will be equal.

Output of the program includes a printer plot of each cross section and the relative amount of discharge at points along the section (Fig. B-3). Dimensions of channels for equal flow in all channels are plotted and output in tabular form to be used as input to INLET (Fig. B-3). A summary table gives the relative friction loss per foot of channel length for each cross section (Table B-2). This table shows which areas of the inlet are most important in dissipating friction; e.g., section 8 has the highest friction loss for the idealized inlet (Table B-2, Fig. B-4).

Table B-3 is a listing of the program NET including comments on program flow.

Output information from NET showing channel location (e.g., Fig. B-3) should be added to each cross section and channels drawn by connecting the channel location points (e.g., points A, B, C, D). If the cross sections are not perpendicular to the computed channel locations in high friction areas of the inlet, these cross sections can be redrawn and channel locations recomputed to obtain a more accurate flow net. However, this should not be necessary in most cases. Point A in Figure B-4 shows that section 1 is not perpendicular to flow and it should be redrawn if used in a model.

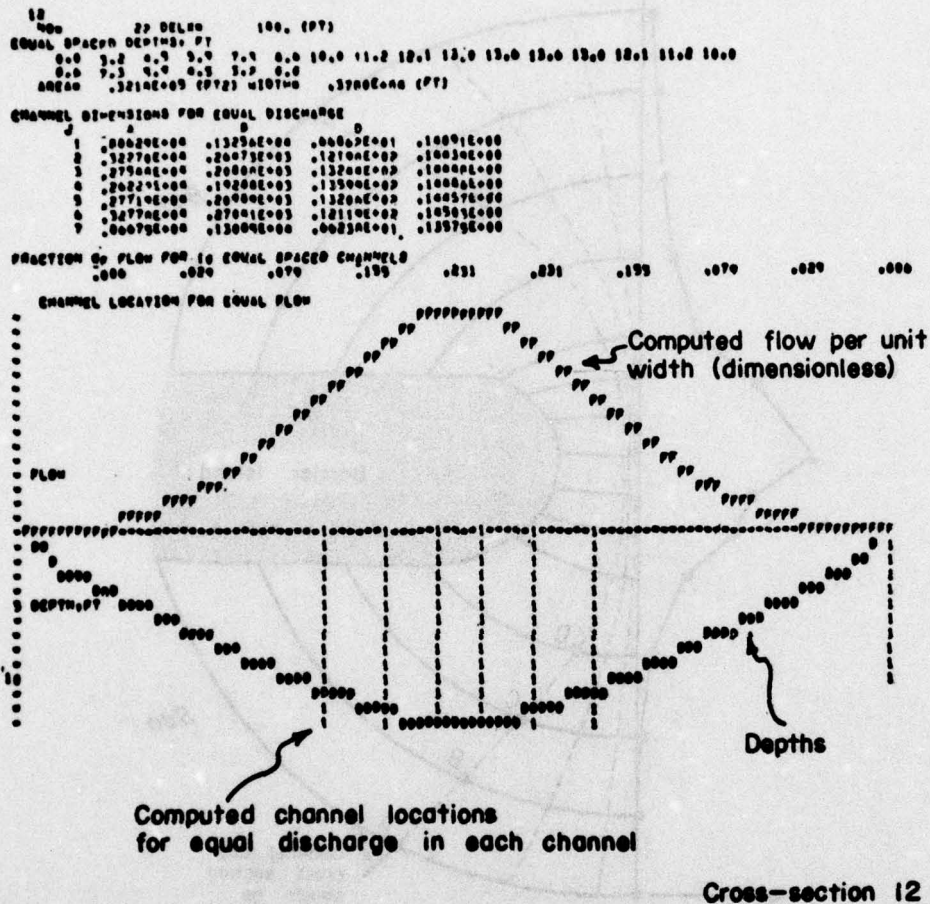


Figure B-3. Computed channel locations for a cross section using minimum friction weighting (output from program NET).



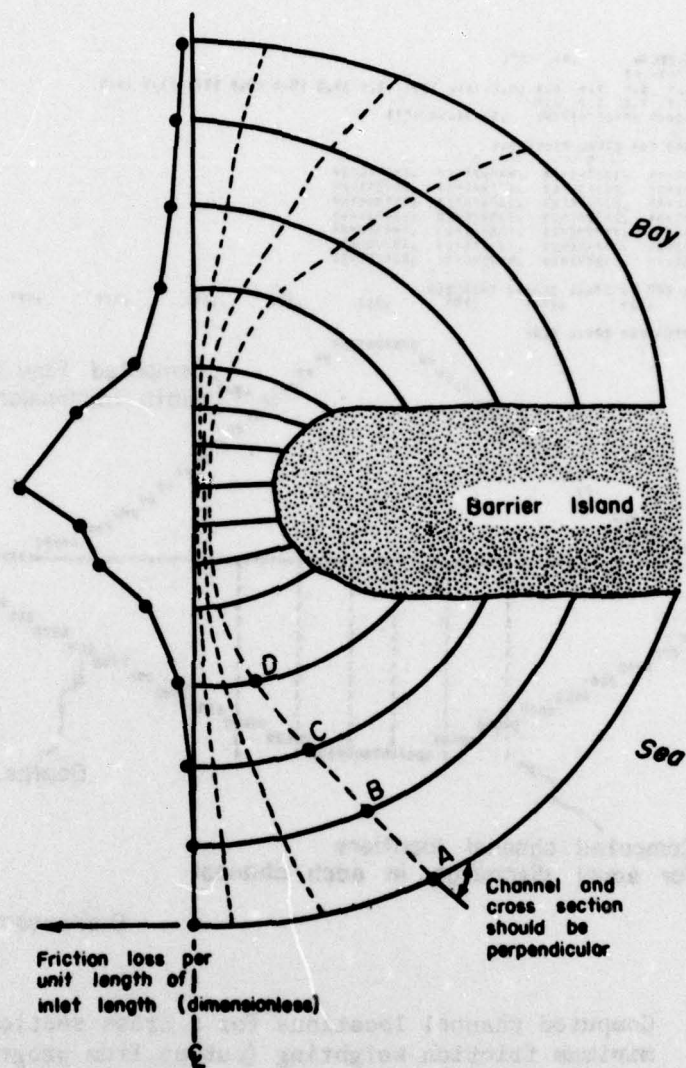


Figure B-4. Idealized inlet flow net and predicted friction loss along the inlet length (adapted from output from the computer program NET).

Table B-1. Input to the computer program NET.

Card	Format	Description
1	I10	Read the number of inlets to be input.
2	I10, F10.0	Read number of channels, inlet discharge (cubic feet per second).
3	I10, F10.0, A4	Read number of depth readings, spacing between depth readings (feet), title of section. Spacing of depth readings should be small enough so that linear interpolation between readings will adequately describe the bottom topography for each segment.
4	16F5.1	Read depths at equal spacing across the channel (feet). The first and last values should be 0.0 (at the waterline at each end of the section). Card type 4 is repeated as many times as necessary to read all the depth values for this section (e.g., for 20 depth readings there would be two card type 4's for that section).

NOTE.--For each cross section for an inlet repeat card types 3 and 4.

Place a blank card after the last cross section of an inlet to indicate the end of that inlet.

For each inlet repeat card types 2, 3, and 4.

Table B-2. Summary of friction losses.

Section	Loss/ft channel length (dimensionless)
1	0.206
2	0.398
3	0.699
4	1.845
5	5.740
6	11.298
7	13.057
8	21.451
9	14.598
10	14.222
11	6.860
12	4.360
13	2.471
14	1.695
15	1.100



Table B-3. Listing of the computer program NET.

PROGRAM NET 74/74 OPT=1 FTH 4.0+420 10/13/77 14.44.05

```

PROGRAM NET(INPUT,OUTPUT,TAPE5=INPUT,TAPE6=OUTPUT)
DIMENSION DP2(2000),C(2000),U(7),B(7),A(7),W(7),NS(7),DP(300)
DIMENSION P(25),FLO=(10),DPT(100),FLO*2(100),IPLOT(102,51)
DIMENSION XC(7),NCUR(7)

C
C PROGRAM TO AID CONSTRUCTION OF INLET FLOW NETS. FLOW IS DISTRIBUTED BY
C ASSUMING THAT FRICTION IS MINIMIZED. SEE SEELIG, HARRIS AND MENCHENRODER,
C 1977, (A GENERALIZED LUMPED PARAMETER MODEL OF INLET HYDRAULICS), U.S.
C ARMY COASTAL ENGINEERING RESEARCH CENTER, KINGMAN BUILDING, FT. BELVOIR,
C VIRGINIA.
C WORK UNIT F31019, GENERALIZED INVESTIGATION OF TIDAL INLETS.
C THIS PROGRAM IS CATALOGED AS 720X6RIAGO(NET) IN THE CERC PROGRAM LIBRARY.
C
C NINLETS = NUMBERS OF INLETS TO BE COMPUTED
  READ(5,1) NINLET
  DO 999 I=1,NINLET
    C IC = NUMBER OF CHANNELS
    C Q = INLET DISCHARGE, CFS
    READ(5,1) IC,Q
    1  FORMAT(110,F10.0,A4)
    WRITE(6,31) IC,Q
    31  FORMAT(1H1,5X,3MIC=,I10,3H Q=,F10.0)
    FSUM=0.
    IS=0
    NT=1.0/FLOAT(IC)
    WRITE(6,71) NT
    71  FORMAT(//,5X,3HHEIGHT OF EACH CHANNEL SHOULD BE = ,F5.2)
    110  CONTINUE
  C FOR EACH CROSS-SECTION READ DEPTH DATA AND DETERMINE CHANNEL GRID SPACING
  C SO THAT FLOW IN EACH CHANNEL IS APPROXIMATELY EQUAL. FLOW IS ASSUMED
  C TO BE DISTRIBUTED TO MINIMIZE FRICTION.
  C
  C READ CONTROL AND DEPTH INFORMATION FOR EACH CROSS-SECTION
  C NO = NUMBER OF DEPTH READINGS ACROSS THE CHANNEL
  C DELX = DISTANCE BETWEEN THE DEPTH READINGS, FT
  C ITITLE = FOUR CHARACTER TITLE OF SECTION
  READ(5,1) NO,DELX,ITITLE
  DO 109 I=1,NO
    109  FLO=(I)=Q.
    DO 1109 I=1,100
      DPT(I)=0.
    1109  FLO*2(I)=0.
    IF(NO,EO,0) GO TO 198
    IS=IS+1
    WRITE(6,332) IS
    332  FORMAT(1H1,2X,13)
    WRITE(6,32) NO,DELX,ITITLE
    32  FORMAT(5X,3HNO=,I10,9H DELX=,F10.0,5H (FT),2X,8HSECTION ,A4)
  C DP = EQUALLY SPACE DEPTH READINGS ACROSS THE CHANNEL, FT
  READ(5,2) (DP(I),I=1,NO)
  2  FORMAT(16F5.1)
  DO 3370 I=1,NO
    3370  DP(I)=DP(I)+2.

```

PROGRAM NET

74/74 OPT1

FTN 4.0+420

10/13/77 14.44.05

```

      WRITE(6,133)
133  FORMAT(2X,24'EQUAL SPACED DEPTH: FT')
      WRITE(6,33) (DP(I),I=1,ND)
33   FORMAT(5X,16F5.1)
      AREA=0.
      NDM=ND+1
      DO 231 I=1,NDM
        J=I+1
231  AREA=AREA+DELX*(DP(I)+DP(J))/2.
      IDTH=DELX*(FLOAT(NDM))
      WRITE(6,232) AREA=IDTH
232  FORMAT(5X,5'AREA= ',E12.4,M (FT2) ,7M IDTH= ',F12.4,M (FT) )

C
C CONVERT EQUAL DISTANCE SPACED DEPTH READINGS TO 2000 EQUAL DISTANCE SPACED
C DEPTH READINGS BY LINEAR INTERPOLATION
      DELX2=DELX*FLOAT(ND+1)/1999.
      DP2(1)=0.
      DP2(2000)=0.
      DO 140 I=2,1999
        D18=FLOAT(I-1)*DELX2
        J1=D18/DELX
        DELD=D18-FLOAT(J1)*DELX
        J1=J1+1
        J2=J1+1
        DP2(I)=DP(J1)*((DP(J2)-DP(J1))/DELX)*DELD
        IF(DP2(I).LE.0.0) WRITE(6,141) I,DP2(I)
141  FORMAT(5X,2MDP2,LE.0 ,2X,13+2X,E12.4)
        IF(DP2(I).LE.0.0) DP2(I)=1.
140  CONTINUE
      CSUM=0.

C
C COMPUTE MEAN DEPTHS AND HEIGHTS OF EACH OF 1999 SECTIONS
      DO 141 I=2,2000
        J=I-1
        DP2(J)=(DP2(I)+DP2(J))/2.
        A1=0.03777-0.000667*DP2(J)
        IF(XN.LT.0.01) XN=0.01
        AREA=DP2(J)*DELX2
        C(J)=AREA*2.0*DP2(J)*0.3333/(XN*2.0*2.0*DELX2)
141  CSUM=CSUM+C(J)
      DO 350 J=1,1999
        C(J)=C(J)/CSUM
        ISUB=(J-1)/200+1
        FLUM=ISUB*FLO=(ISUB)*C(J)
C SAVE INFORMATION FOR 100 GRID8 ACROSS FOR A PRINTER PLOT
        ISUB2=(J-1)/20+1
        FLUM2=ISUB2*FLO=2*(ISUB2)*C(J)*100.
        OPT(ISUB2)=OPT(ISUB2)+DP2(J)/19.99
350  CONTINUE
C COMPUTE CHANNEL DIMENSIONS SO THAT EACH CHANNEL HAS EQUAL DISCHARGE.
      DO 1142 I=1,7
        AB(I)=0
        A(I)=0.

```



PROGRAM NET

78/78 OPT=1

FTN 8.6+420

10/13/77 18.48.05

```

      N(I)=0.
1142 B(I)=0.
      C FIRST ESTIMATE
      J=1
      DO 142 I=1,1000
        NB(J)=NB(J)+1
        A(J)=A(J)+DP2(I)*DELX2
        B(J)=B(J)+DELX2
        N(J)=N(J)+C(I)
        IF(N(J).GT.NT) J=J+1
        IF(J.GT.IC) WRITE(6,134)
134  FORMAT(5X,7HJ ERROR )
142  CONTINUE
      C ITERATE TO ESTIMATE CHANNEL LOCATIONS
      NIT=50
      DO 1010 IPR=1,NIT
        CC6=0.
        DO 241 J=1,IC
          D(J)=A(J)/B(J)
          XN=0.0377-0.000667*D(J)
          IF(XN.LE.0.01) XN=0.01
          N(J)=A(J)**2.*D(J)**0.3333/(XN**2.*B(J))
241  CC6=CC6+N(J)
        DO 242 J=1,IC
          N(J)=N(J)/CC6
242  N(J)=N(J)/CC6
      C DETERMINE NUMBER OF WIDTHS TO CORRECT EACH CHANNEL
      C CASE IS TAKEN TO CORRECTLY ROUND THE NUMBERS TO OBTAIN 1999 WIDTH CELLS
      XMAX=1000000.
      XMIN=1000000.
      ERROR=0.
      DO 2234 J=1,IC
        ER=ABS(N(J)-NT)*100.
        IF(ER.GT.ERROR) ERROR=ER
        XC(J)=(N(J)-NT)*1999./0.2
        IF(XC(J).GT.XMAX) XMAX=XC(J)
        IF(XC(J).GT.XMAX) XMAX=XC(J)
        IF(XC(J).LT.XMIN) XMIN=XC(J)
2234 IF(XC(J).LT.XMIN) XMIN=XC(J)
      NCC=0
      DO 2235 J=1,IC
        NCCOR(J)=XC(J)
2235 NCC=NCC+NCCOR(J)
        IF(NCC.LT.0) NCCOR(JMIN)=NCCOR(JMIN)-NCC
        IF(NCC.GT.0) NCCOR(JMAX)=NCCOR(JMAX)+NCC
        I=0
        DO 2236 J=1,IC
          A(J)=0.
          B(J)=0.
          NB(J)=NB(J)-NCCOR(J)
          IF(NB(J).LT.1) NB(J)=1
          NNB=NB(J)
        DO 223 L=1,NB
          I=I+1

```

```

      IF(I.GT.1999) GO TO 233
      A(J)=A(J)+DP2(I)*DELX2
      B(J)=B(J)+DELX2
233  CONTINUE
234  CONTINUE
      IF(ERROR.LT.0.3) GO TO 1011
1010 CONTINUE
C WRITE CHANNEL DIMENSIONS
1011 WRITE(6,361)
361  FORMAT(/,2X,30HCHANNEL DIMENSIONS FOR EQUAL DISCHARGE )
      WRITE(6,1123)
1123 FORMAT( 1X,1HJ,0X,1HA,12X,1HB,12X,1HD)
      DO 146 J=1,IC
      WRITE(6,147) J,A(J),B(J),D(J),W(J)
147  FORMAT(5X,15.0E12.3)
146  CONTINUE
C COMPUTE AND WRITE THE FRACTION OF FLOW IF THE INLET IS DIVIDED INTO 10
C EQUAL SPACED CHANNELS.
      WRITE(6,362)
362  FORMAT(/,2X,45HFRACTION OF FLOW FOR 10 EQUAL SPACED CHANNELS )
      WRITE(6,367) (FLOW(IP),IP=1,10)
367  FORMAT( 5X,10F10.3)
C PRINT PLOT DEPTHS AND PREDICTED RELATIVE DISCHARGE ACROSS THE CHANNEL
      DO 601 I=1,102
      DO 602 J=1,51
      IPLOT(I,J)=1H
602  CONTINUE
601  CONTINUE
      DO 603 I=2,102
      IPLOT(I,10)=1H
      DO 604 J=1,51
      IPLOT(I,J)=1H
604  CONTINUE
      DO 609 J=1,IC
      NNN= (10+NB(J))/20
      IF(NN.GT.102) NN=102
      DO 610 JJ=10,51
      IPLOT(NN,JJ)=1H
610  CONTINUE
      FMAX=0.
      DO 612 I=1,100
      IF(FLOW2(I).GT.FMAX) FMAX=FLOW2(I)
      IDMAX=16
      DO 605 I=1,100
      IID=1+2
      ID=IDPT(I)+16
      IF(ID.LT.1.OR.ID.GT.51) ID=51
      IF(ID.GT.IDMAX) IDMAX=ID
      IPLOT(IID,ID)=1H
      ID=17+FLOW2(I)/FMAX*16.
      IF(ID.LT.1.OR.ID.GT.51) ID=1
605  IPLOT(IID,ID)=1H
      WRITE(6,608)

```



PROGRAM NET

74/74

OPT=1

FTN 4.6+420

10/13/77 14.44.05

```

608 FORMAT(/,5X,31HCHANNEL LOCATION FOR EQUAL FLOW )
IPLOT(2,21)=1M5
IPLOT(1,26)=1M1
IPLOT(2,26)=1M0
IPLOT(1,31)=1M1
IPLOT(2,31)=1M5
IPLOT(1,36)=1M2
IPLOT(2,36)=1M0
IPLOT(1,41)=1M2
IPLOT(2,41)=1M5
IPLOT(1,46)=1M3
IPLOT(2,46)=1M0
IPLOT(1,51)=1M3
IPLOT(2,51)=1M5
IPLOT(4,12)=1MF
IPLOT(5,12)=1ML
IPLOT(6,12)=1M0
IPLOT(7,12)=1MH
IPLOT(8,21)=1M0
IPLOT(5,21)=1ME
IPLOT(6,21)=1MP
IPLOT(7,21)=1MY
IPLOT(8,21)=1MH
IPLOT(9,21)=1M
IPLOT(10,21)=1MP
IPLOT(11,21)=1MY
DO 606 J=1,10MAX
606 WRITE(6,607) (IPLOT(I,J),I=1,102)
607 FORMAT(1X,102A1)
F(18)=0.
DO 601 J=1,10
601 J=1,10
XN=0.03777-0.000067*D(J)
F(18)=F(18)+XN*XN*D(J)/(A(J)**2.*D(J)**0.3333)
601 CONTINUE
FSUM=FSUM+F(18)
GO TO 110
108 CONTINUE
C WRITE A SUMMARY TABLE OF THE RELATIVE FRICTION LOSS PER FOOT OF CHANNEL
C LENGTH FOR ALL SECTIONS OF THE INLET.
WRITE(6,106)
106 FORMAT(/,5X,26HSUMMARY OF FRICTION LOSSES //,2X,7HSECTION ,
1 5X,36HLOSS/FT CHANNEL LENGTH (DIMENSIONLESS) //)
DO 107 I=1,10
F(1)=F(1)/FSUM*100.
WRITE(6,109) I,F(1)
109 FORMAT(6X,12,12X,F0.3)
107 CONTINUE
999 CONTINUE
STOP
END

```

A suggested flow net grid system which includes important friction zones of the idealized inlet is shown in Figure B-5.

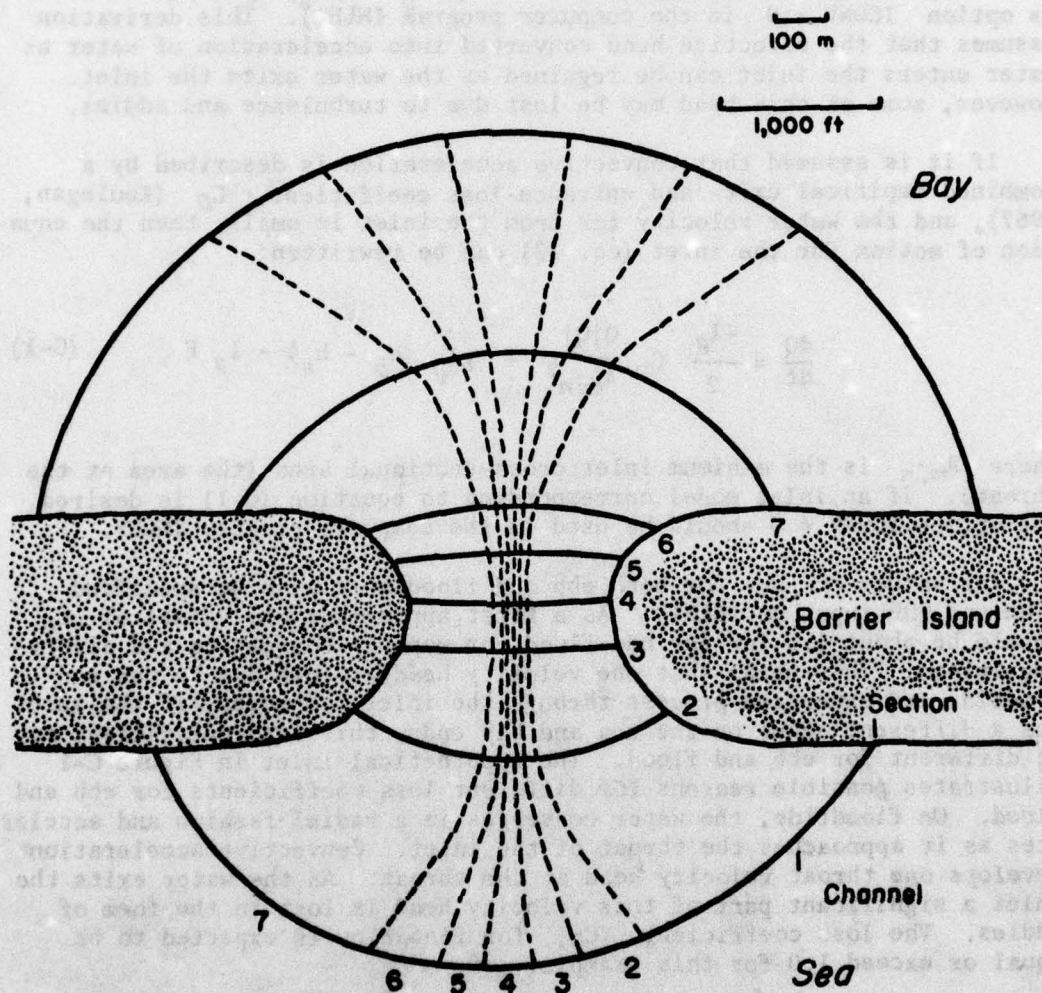


Figure B-5. A suggested grid cell system for the idealized inlet (seven channels and eight cross sections).



## APPENDIX C

### ALTERNATIVE DEVELOPMENT OF THE CONVECTIVE ACCELERATION TERM

In equation (1) it was assumed that the convective acceleration term could be integrated along the length of the inlet. The resulting expression for convective acceleration is  $1/2 (1/A_D^2 - 1/A_8^2)$  (see eq. 6, this is option ICONV = 0 in the computer program INLET). This derivation assumes that the effective head converted into acceleration of water as water enters the inlet can be regained as the water exits the inlet. However, some of this head may be lost due to turbulence and eddies.

If it is assumed that convective acceleration is described by a combined empirical exit- and entrance-loss coefficient,  $C_D$  (Keulegan, 1967), and the water velocity far from the inlet is small, then the equation of motion for the inlet (eq. 12) can be rewritten:

$$\frac{dQ}{dt} = \frac{-I_g}{2} C_D \frac{Q|Q|}{A_{min}^2} - g I_g (h_b - h_s) - I_g F \quad (C-1)$$

where  $A_{min}$  is the minimum inlet cross-sectional area (the area at the throat). If an inlet model corresponding to equation (C-1) is desired, the option ICONV  $\neq$  0 should be used in the computer program INLET.

The values of  $C_D$  for both ebb and flood must also be specified (CDE and CDF input to INLET). As a first approximation, values of  $C_D$  should be about 1.0 for ebb and flood for rapidly converging and diverging inlets. This means that one velocity head is lost due to the contraction and expansion process through the inlet. However, if the inlet has a different shape on the sea and bay ends, this loss coefficient may be different for ebb and flood. The hypothetical inlet in Figure C-1 illustrates possible reasons for different loss coefficients for ebb and flood. On floodtide, the water converges in a radial fashion and accelerates as it approaches the throat of the inlet. Convective acceleration develops one throat velocity head at the throat. As the water exits the inlet a significant part of this velocity head is lost in the form of eddies. The loss coefficient,  $C_D$ , for floodflow is expected to be equal or exceed 1.0 for this example.

On the ebbtide, water approaches the inlet in an approximately radial fashion and then diverges slowly due to the funnel shape of the seaside of the inlet. This pattern of flow causes little head loss due to eddies, so a significant part of head converted into acceleration of water on entrance is regained as the flow exits the inlet. The value of  $C_D$  for this flow condition is expected to be less than 1.0.

As a first approximation values of  $C_D$  for ebb and flood can be taken as 1.0 for rapidly converging inlets. These loss coefficients can

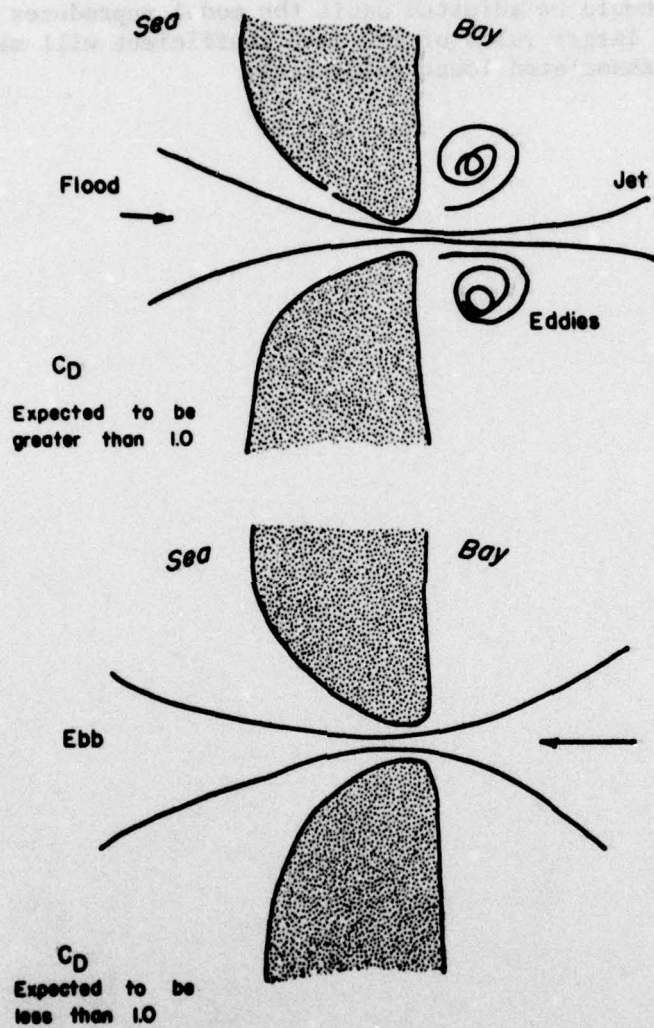


Figure C-1. Flood and ebb convective losses for a hypothetical inlet.



then be adjusted during calibration to account for differences in ebb flow and floodflow. For example, if in the prototype flood velocities are lower than ebb velocities, the values of the ebb- and flood-loss coefficients should be adjusted until the model reproduces observed velocities. A larger value of the loss coefficient will mean a greater head loss and associated lower velocities.

## APPENDIX D

### COMPUTER PROGRAM (INLET) DOCUMENTATION

#### 1. Program Description.

The numerical model to predict inlet hydraulics described in the text is programed in FORTRAN for a CDC 6600 computer. The simultaneous differential equations are solved using a variable time step Runge-Kutta-Gill marching procedure. The organization of the computer program is shown in Figure D-1. The following is a brief description of each routine:

INLET is the main routine which controls input-output and calls sub-routines to execute a specific task. Figure D-1 summarizes control throughout the program. Variable names in the program corresponding to symbols used in the text are given in Symbols and Definitions. The program is organized to accept up to three inlets connecting the bay to the sea, up to seven channels for each inlet, and up to eight cross sections (seven grids long).

Subroutine HELM uses an iterative method of estimating the natural pumping period or Helmholtz period,  $T_H'$ , for the inlet-bay system by neglecting friction in the inlet to give

$$T_H' = 2\pi \sqrt{\frac{(L_{in} + L') A_{bay}}{gA_c}}, \quad (D-1)$$

where  $L'$  is added inlet length due to radiation, and given by:

$$L' = \frac{-B}{\pi} \ln \left( \frac{\pi B}{\sqrt{gd} T_H} \right) \quad (D-2)$$

Subroutine RKGS is a routine to solve simultaneous differential equations. This subroutine was adapted from the Scientific Subroutine Package (International Business Machines, 1970).

Subroutine SETEQ evaluates the right-hand side of the equation of motion, one for each inlet, and the continuity equation between the inlet and bay for each time step. This routine also evaluates the relative rank of the four terms in the equation of motion for flow in each inlet.

Subroutine LEVEL determines the water level in the grids at each time step. The routine interpolates the level between the sea and bay based on the relative amount of friction in each grid cell.



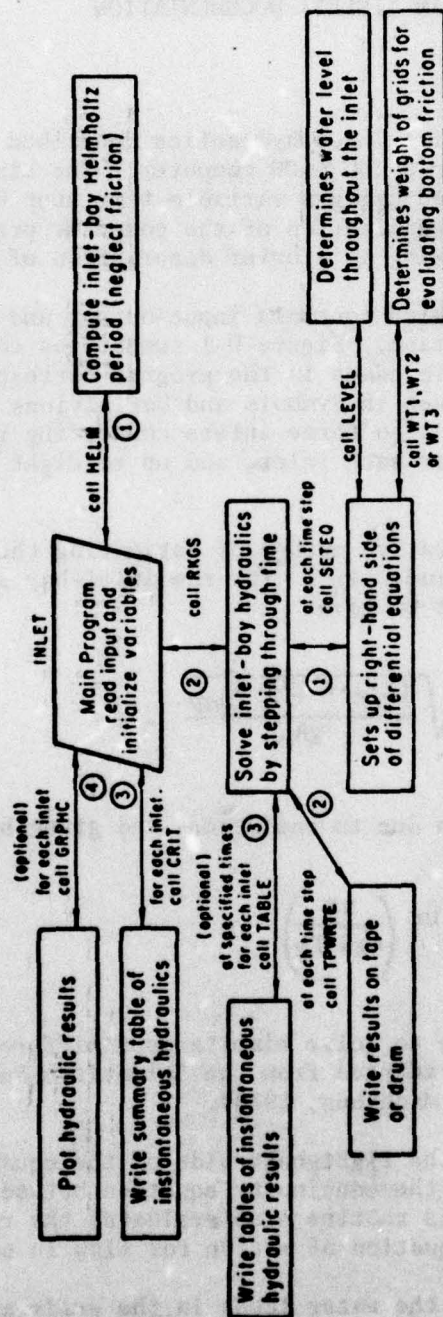


Figure D-1. Flow chart of the computer program (INLET).

Subroutine TPWRTE writes hydraulic results from each time step on a tape or disc, so that this information can be used later by the output routines.

Subroutine TABLE outputs a table of instantaneous hydraulics each time the routine is called.

Subroutine SEA determines the water level in the sea as a function of time either for a given sine wave or by interpolating equal time-series data.

Subroutine WT1 determines the grid weighting function by assuming that the flow is distributed across each section so that friction is minimized. This routine allows flow to cross channel boundaries, but assumes that this flow will be small, so the flow is neglected in the equation of motion. See Appendix A for a derivation of the procedure used in this routine.

Subroutine WT2 is similar to WT1, except that flow is not allowed to cross channel boundaries, and the flow is distributed in each channel so that friction is minimized.

Subroutine WT3 determines the weighting function so that flow is distributed equally in all grids. This is generally unrealistic, since it will be difficult to visually draw this grid system. However, this routine is useful since it provides an upper limit on frictional effects and therefore gives a lower limit of bay levels and inlet velocities. This weighting can be used to model simple geometry inlets where only one channel is used to represent the inlet.

Subroutine CRIT prints a table of critical instantaneous hydraulics (i.e., at high water, low water, maximum velocity, and maximum discharge). This table is determined by storing a summary of conditions for each time step, then scanning this list for critical values (routine adapted from Huval and Wintergerst, 1977).

Subroutine GRPHC plots mean inlet hydraulics by scaling hydraulics in storage and plotting the time interval requested on a digital x-y pen plotter (adapted from Huval and Wintergerst, 1977).

Subroutine READIN is used by GRPHC to read data in storage and scale values for plotting (adapted from Huval and Wintergerst, 1977).

## 2. Input.

The computer program INLET requires the following input, one deck for each inlet-bay system:



<u>Card Type</u>	<u>Variables</u>	<u>Format</u>	<u>Description</u>
1	ALABL1	4A10	First line of title
	ALABL2	4A10	Second line of the title
2		5I10, 2F10.5, I10	
	NINLET		Number of inlets
	IPLOT		=1 for plot of results
	IWT		Weighting type =1 flow distributed to minimize friction at each section =2 flow distributed in each channel to minimize friction =3 equal flow in channels
	ITABLE		=1 for tables of instantaneous hydraulics
	C1, C2		Manning's n evaluated by: $n = C1 - C2 * D$ ; where D is stillwater depth. If blank, default values of C1 = 0.03777 and C2 = 0.000667 are assumed.
	ICONV		ICONV = 0 for convective acceleration as derived in the text ICONV $\neq$ 0 for alternative derivation (see App. C).
3		3F10.5, E10.4, 3F10.5, 2F5.1	
	T		Forcing period, hours
	DELT		Approximate time increment
	AO		Forcing wave amplitude (feet)
	AB		Bay area at datum (square feet)
	BETA		Bay area variation parameter
	ZETA		Inlet side slope $D(z)/D(y)$

<u>Card Type</u>	<u>Variables</u>	<u>Format</u>	<u>Description</u>
	QINFLO		Bay inflow from sources other than the inlet (cubic feet per second)
	CDF		An empirical flood-loss coefficient
	CDE		An empirical ebb-loss coefficient
4		2I10, F10.0	
	IC		Number of channels
	IS		Number of cross sections
	QINT		Estimated inlet discharge at the time the model starts
5	(one card per section)	10X, 7F10.5	
	A'		Cell cross-sectional areas at the ends of each cell at datum (square feet) (see Fig. B-2)
6	(one card per section)	10X, 7F10.5	
	B'		Grid cell widths for the end of each cell (feet) (see Fig. 2)
7	(one less card than sections)	10X, 7F10.5	
	L'		Lengths of the sides of cells (see Fig. 2) (one less card than number of sections, one more value per card than the number of channels)

For card types 5, 6, and 7 there will be one card for each cross section of the inlet. The first card will be for the first cross section, i.e., the section closest to the sea; the last section is adjacent to the bay. The first value on each card will correspond to the first channel



<u>Card Type</u>	<u>Variables</u>	<u>Format</u>	<u>Description</u>
which is adjacent to land, and the last value on a card will be for the last channel, which is also adjacent to land (Fig. 2).			
(FOR MORE THAN ONE INLET CONNECTING THE BAY TO THE SEA REPEAT CARD TYPES 3 TO 7 FOR EACH ADDITIONAL INLET).			
8	TDEL	34X, F6.2	Water level sampling interval (minutes)
	NPTS	6X, I3	Number of sample points = 0 for no data
9	(optional-no cards if NPTS = 0 from card type 8)		
	Y		Eight water level values per card, as many cards to include NPTS points; start the model at a time when the sea level is zero. Use 25 or more points per forcing cycle for best results (i.e., levels at 30- or 15-minute intervals for a 12-hour tide).
10	(optional-two plot cards, first card used only if IPLOT = 1 on card type 1)		
	8F10.5/,3F10.5, I10		
	XO		Starting time of plot (hours)
	XF		Ending time of plot (hours)
	SCALX		Time scale (hours per inch)
	YLO		Minimum value of water levels (feet)
	YL		Overall height of plot (inches)
	YLSCAL		Scale of water level height (feet per inch)
	YRO		Minimum flows (thousands cubic feet per second)
	YRSCAL		Scale of flows (thousands cubic feet per second per inch)

<u>Card Type</u>	<u>Variables</u>	<u>Format</u>	<u>Description</u>
Second card	YVO		Minimum velocity (feet per second)
	YVSCAL		Scale of velocities (feet per second per inch)
	SCALE		Scale factor for total plot size
	IQ		IQ = 0 for no plot of inlet discharge

11 If a plot is requested, repeat card types 8 and 9 for observed bay levels to compare with predictions (card type 8 required; use NPTS = 0 for no observed bay levels). Only one set of card types 10 and 11 will be required for plotting even though the system modeled may have more than one inlet.

12 End of file card.

The inlet data for a computer run of Masonboro Inlet are shown in Figure D-2.

### 3. Output.

The types of output include (a) a summary table of grid dimensions, input parameters, and the Helmholtz period of the system estimated, assuming there is no friction in the inlet; (b) (optional) summary tables of instantaneous inlet hydraulics; (c) (optional) a pen plot of inlet hydraulics; and (d) a table summarizing critical points throughout model operation, such as high water, low water, point of maximum discharge, and maximum velocity. Samples of input and output for the Masonboro run are given in Figures D-3, D-4, and D-5. The computer plot is included in the text (Fig. 15).

### 4. Computer Program.

A listing of the computer program INLET follows the sample output (Table D-5). The program was written in FORTRAN IV for a CDC 6600 computer with plotter. Control cards, plotting instructions, and file controls may have to be changed for other computers. If no plotter is available, the subroutine GRPHC and the call to the subroutine in the main program may be removed.



```

MASONBORO 1469
COF=2,
25.0 1 1 1 2.15 1 2.0000E+09 0.2 1 0. 0. 2. 0.
      4 7 -20000.
A1 28280. 5510. 4570. 2420.
A2 9725. 7885. 5680. 2140.
A3 1080. 5450. 4625. 3780.
A4 940. 2525. 10030. 5285.
A5 500. 1030. 5079. 4080.
A6 1770. 5850. 4330. 3925.
A7 4790. 6810. 4400. 4000.
B1 1000. 680. 260. 90.
B2 1320. 1400. 310. 100.
B3 500. 1380. 280. 240.
B4 150. 430. 450. 500.
B5 280. 150. 280. 350.
B6 800. 890. 420. 440.
B7 800. 670. 470. 240.
L1 850. 900. 1000. 1000. 1000.
L2 750. 950. 1000. 1000. 1000.
L3 400. 550. 900. 1050. 1200.
L4 500. 700. 850. 900. 900.
L5 400. 800. 950. 600. 200.
L6 2800. 2100. 2100. 3600. 3400.
GAGE0 9/12/40 MASONBORO DELTA 30. NUM= 50
-1.39 -1.40 -1.65 -1.60 -1.38 -0.98 -0.80 -0.08
0.34 0.82 1.29 1.70 2.08 2.33 2.48 2.50
2.41 2.22 1.91 1.50 1. 0.50 0. -0.50
-0.08 -1.32 -1.55 -1.62 -1.60 -1.44 -1.03 -0.69
-0.20 0.36 0.93 1.40 1.74 2.10 2.31 2.40
2.48 2.29 1.97 1.56 1.14 0.6 0.1 -0.4
-0.9 -1.3 2. -3. 0 6. 1. -6. 20.
0. 22. 1.
-6. 2.
NO BAY
END

```

Figure D-2. Sample of input data for a computer run of Masonboro Inlet, North Carolina.

-----  
 MASONBORO 1969  
 TEST

CONTROL CARDS  
 1 0 2 1 0.00000 0.00000 1  
 25.00000 200.00000 2.15000 20000.00 .20000 .01330 0.00000 2.0 0.0

SUMMARY OF INLET GRID CHARACTERISTICS  
 INLET NUMBER 1

SECTION 1  
 CHANNEL # 1 2 3 4  
 AREA(FT<sup>2</sup>) 19002.5 4697.5 5125.0 2280.0  
 WIDTH(FT) 2160.0 1680.0 264.0 .95.0  
 DEPTH(FT) 8.60 6.44 17.98 24.00  
 LEN(FT) 875.0 950.0 1000.0 1000.0  
 N .0319 .0335 .0254 .0210

SECTION 2  
 CHANNEL # 1 2 3 4  
 AREA(FT<sup>2</sup>) 6402.5 6767.4 5452.5 2920.0  
 WIDTH(FT) 910.0 1390.0 295.0 180.0  
 DEPTH(FT) 7.00 4.47 18.16 16.72  
 LEN(FT) 850.0 975.0 1000.0 1000.0  
 N .0331 .0345 .0250 .0269

SECTION 3  
 CHANNEL # 1 2 3 4  
 AREA(FT<sup>2</sup>) 2010.0 4087.4 7827.5 4492.5  
 WIDTH(FT) 425.0 905.0 365.0 400.0  
 DEPTH(FT) 4.73 4.52 21.45 11.23  
 LEN(FT) 495.0 725.0 974.0 1125.0  
 N .0346 .0348 .0235 .0303

SECTION 4  
 CHANNEL # 1 2 3 4  
 AREA(FT<sup>2</sup>) 720.0 2780.5 7554.5 4682.5  
 WIDTH(FT) 315.0 290.0 365.0 445.0  
 DEPTH(FT) 2.29 9.59 20.70 10.52  
 LEN(FT) 600.0 775.0 874.0 900.0  
 N .0362 .0314 .0240 .0308

SECTION 5  
 CHANNEL # 1 2 3 4  
 AREA(FT<sup>2</sup>) 2135.0 4683.0 5204.5 4002.4  
 WIDTH(FT) 560.0 920.0 358.0 405.0  
 DEPTH(FT) 3.81 8.54 14.87 9.48  
 LEN(FT) 600.0 875.0 774.0 400.0  
 N .0352 .0321 .0279 .0312

SECTION 6  
 CHANNEL # 1 2 3 4  
 AREA(FT<sup>2</sup>) 4080.0 6230.0 6864.0 3902.4  
 WIDTH(FT) 910.0 780.0 545.0 360.0  
 DEPTH(FT) 4.48 7.99 12.90 11.01  
 LEN(FT) 2350.0 2100.0 2850.0 3800.0  
 N .0348 .0324 .0294 .0304

FORCING PERIOD= 25.00 HOURS  
 TIME(LAPPROX)= 5.17 HOURS  
 TF/TMO 7.88  
 INLET LENGTH ADDED LENGTH  
 1 1622.5 1749.4

TDEL= MINS 30.00 NPTS= 50

-1.39 -1.60 -1.65 -1.60 -1.38 -.98 -.60 -.08 .34 .62 1.29 1.70 2.08 2.33 2.48 2.50  
 2.41 2.22 1.91 1.50 1.00 .50 0.00 -.50 -.98 -1.32 -1.55 -1.62 -1.60 -1.48 -1.03 -.69  
 -.20 .36 .93 1.40 1.74 2.10 2.31 2.49 2.48 2.29 1.97 1.56 1.16 .60 .10 -.40  
 -.00 -1.38

Figure D-3. Sample output from INLET (summary table for Masonboro Inlet input data).



```

-----
TIME: HOURS = 6.000 DELT. SEC = 400.00

INLET 1
SEA LEVEL+FT= 2.00
SAV LEVEL+FT= 1.23
DISCHARGE+CFS= .5481E+04
DAY AREA = .2493E+09 FT2

CHANNEL SECTION 1 2 3 4 5 6 7 FRICTION
FRIC .04 .06 .07 .42 .11 .31 .12

1
1 LEVEL 2.00 2.00 2.06 1.70 1.32 1.26
1 V(FPS) .12 .33 .04 2.14 .96 .53
1 Q(CFS) 2802. 2802. 2802. 2802. 2802. 2802.
1 HEIGHT .05 .05 .05 .05 .05 .05
1 FRIC .00 .00 .00 .10 .01 .01 .19

2
2 LEVEL 2.00 2.02 1.94 1.66 1.39 1.29
2 V(FPS) 1.01 .93 1.52 2.71 1.73 1.28
2 Q(CFS) 8993. 8993. 8993. 8993. 8993. 8993.
2 HEIGHT .10 .10 .10 .10 .10 .10
2 FRIC .01 .01 .02 .10 .02 .03 .46

3
3 LEVEL 2.06 2.00 1.95 1.83 1.67 1.42
3 V(FPS) 5.40 4.98 3.63 3.77 5.35 4.07
3 Q(CFS) 31238. 31238. 31238. 31238. 31238. 31238.
3 HEIGHT .57 .57 .57 .57 .57 .57
3 FRIC .03 .03 .02 .11 .07 .20 .23

4
4 LEVEL 2.07 2.04 1.98 1.75 1.54 1.37
4 V(FPS) 4.60 3.50 2.20 2.13 2.42 2.42
4 Q(CFS) 11772. 11772. 11772. 11772. 11772. 11772.
4 HEIGHT .21 .21 .21 .21 .21 .21
4 FRIC .00 .01 .02 .10 .01 .00

TEMP ACC= .06 CONV ACC= 32.4 HEAD= 100.0 FRIC= 67.0
MEAN VELOCITY AT THE MINIMUM AREA SECTION= 2.97 FT/SEC AREA= 18429.73 FT2
-----

```

Figure D-4. Sample output from INLET (summary table of instantaneous hydraulics for Masonboro after 6 hours of model time).

SUMMARY TABLE OF HYDRAULICS INLET 1					
TIME HRS	HS FT	INFLW KCF/S	HS FT	VEL FPS	Q KCF/S
0.334	-1.406	0.000	-0.239	-3.8610	-55.1660
1.056	-1.650	0.000	-0.951	-2.919	-10.568
2.167	-1.303	0.000	-1.5620	0.033	0.033
3.034	0.155	0.000	-0.541	2.4030	37.907
3.945	0.245	0.000	-0.456	2.4810	10.631
5.167	1.386	0.000	0.516	2.9220	90.206
5.300	1.460	0.000	0.690	2.9400	41.646
5.500	1.656	0.000	0.788	2.9450	42.193
5.611	1.744	0.000	0.780	2.9480	52.656
5.723	1.834	0.000	0.967	2.9570	43.252
5.834	1.922	0.000	1.056	2.9600	43.884
5.945	2.005	0.000	1.145	2.9760	44.441
6.056	2.000	0.000	1.234	2.9740	44.806
6.167	2.146	0.000	1.321	2.9540	50.8890
7.300	2.5040	0.000	2.147	2.154	41.977
8.300	2.296	0.000	2.4620	0.086	1.714
10.611	0.444	0.000	1.191	-3.108	-45.7340
10.667	0.309	0.000	1.146	-3.3370	-45.713
10.774	0.278	0.000	1.055	-3.3620	-45.607
10.800	0.166	0.000	0.962	-3.3820	-45.425
11.000	0.054	0.000	0.869	-3.3940	-45.177
11.111	-0.056	0.000	0.774	-3.4110	-44.870
11.223	-0.166	0.000	0.679	-3.4220	-44.519
11.334	-0.279	0.000	0.582	-3.4290	-44.126
11.445	-0.391	0.000	0.485	-3.4330	-43.680
11.556	-0.500	0.000	0.387	-3.4330	-43.170
11.667	-0.611	0.000	0.286	-3.4300	-42.606
11.778	-0.723	0.000	0.188	-3.4270	-42.037
11.889	-0.831	0.000	0.087	-3.4200	-41.412
12.000	-0.933	0.000	-0.018	-3.4030	-40.657
13.723	-1.6250	0.000	-1.618	-1.764	-22.758
14.645	-1.895	0.000	-1.6650	0.073	-0.923
15.300	-0.812	0.000	-1.245	1.8800	25.949
17.274	1.153	0.000	0.185	2.9960	40.070
17.300	1.257	0.000	0.203	3.0200	52.006
17.500	1.354	0.000	0.382	3.0360	42.805
17.667	1.484	0.000	0.528	3.0490	43.6800
17.778	1.459	0.000	0.625	3.0620	43.685
17.834	1.595	0.000	0.72	3.064	43.7200
17.890	1.630	0.000	0.719	3.0330	53.719
18.056	1.740	0.000	0.858	2.994	43.4420
18.111	1.780	0.000	0.904	2.9730	43.408
18.223	1.864	0.000	0.994	2.9650	43.749
18.334	1.949	0.000	1.083	2.9670	50.204
18.445	2.030	0.000	1.172	2.9690	48.648
18.556	2.100	0.000	1.260	2.962	48.8830
19.774	2.5080	0.000	2.099	2.267	44.163
20.723	2.196	0.000	2.4160	-0.016	-0.312
21.778	1.390	0.000	1.904	-2.9040	-42.6280
21.800	1.305	0.000	1.877	-2.9210	-42.545
22.000	1.211	0.000	1.750	-2.982	-52.4770
22.778	0.373	0.000	1.157	-3.3940	-46.6390
22.800	0.264	0.000	1.064	-3.4150	-50.4700
23.000	0.154	0.000	0.970	-3.4290	-50.184
23.111	0.044	0.000	0.876	-3.4400	-49.836
23.223	-0.067	0.000	0.780	-3.4490	-49.460
23.334	-0.178	0.000	0.684	-3.4560	-49.064
23.445	-0.289	0.000	0.587	-3.4590	-48.588
23.556	-0.400	0.000	0.489	-3.4610	-48.092
23.667	-0.513	0.000	0.390	-3.4610	-47.576
23.778	-0.624	0.000	0.290	-3.4630	-47.043
23.889	-0.731	0.000	0.189	-3.4620	-46.510
24.000	-0.849	0.000	0.087	-3.4540	-45.870
24.111	-0.951	0.000	-0.015	-3.4350	-45.003
24.223	-1.052	0.000	-0.117	-3.4090	-44.167
25.000	-1.3900	0.000	-0.855	-2.599	-35.948

\* CRITICAL POINT VALUE

Figure D-5. Sample output from INLET (table of critical points for the model time: high water, low water, etc., for Masonboro Inlet).



Table D-1. Listing of the computer program INLET.

PROGRAM INLET(INPUT,OUTPUT,TAPE5=INPUT,TAPE6=OUTPUT,TAPE9,TAPE10,	INLET	2
1 TAPE3,PUNCH=TAPE3)	INLET	3
C PROGRAM NUMBER 720X6R1880 (INLET) ANALYSES AND PREDICTS INSTANTANEOUS INL	INLET	4
C HYDRAULICS USING A LUMPED PARAMETER SCHEME (SEE SFELIG, HARRIS AND	INLET	5
C MERCHENRODER, 1976, (A GENERALIZED LUMPED PARAMETER MODEL OF INLET	INLET	6
C HYDRAULICS), A DRAFT CERC REPORT)	INLET	7
REAL L,LENGTH,LIN,LX,N,NX	INLET	8
COMMON/NUM5/NI,G,NINLET,ICH(3),ISF(3),OR,L(7,7),B(7,7),D(7,7),	INLET	9
1 A(7,7),N(7,7),W(7,7),V(7,7),Q(7,7),MS,MB,H(7,7),IC,IS,AMINI(3),	INLET	10
1BMINI(3),LIN,OX(3),QINFLO,ARAY,LENGTH(3)	INLET	11
COMMON/NUM1/Y(5),DERV(5),X,INT,ZETA,WM	INLET	12
COMMON/NUM2/BX(3,7,7),DX(3,7,7),MX(3,7,7),WX(3,7,7),LX(3,7,7),NX(3	INLET	13
1,7,7)	INLET	14
COMMON /NUM3/AD,T,AR,RETA	INLET	15
COMMON/NUM4/RNK(3,4)	INLET	16
DI=FN\$ION CORL(3)	INLET	17
DIMENSION ALABL1(4),ALABL2(4),IBUF(1000),NUMBER(20)	INLET	18
3370 CONTINUE	INLET	19
DO 2193 II=1,3	INLET	20
2193 OX(II)=1.	INLET	21
C GO ACCELERATION OF GRAVITY	INLET	22
G=32.2	INLET	23
DO 1211 I=1,20	INLET	24
1211 NUMLEN(I)=1	INLET	25
WRITE(6,2937)	INLET	26
2937 FORMAT(//,1X,{-----}{	INLET	27
READ(5,1167) (ALABL1(I),I=1,4)	INLET	28
READ(5,1167) (ALABL2(I),I=1,4)	INLET	29
1167 FORMAT(4A10)	INLET	30
WRITE(6,1168) (ALABL1(I),I=1,4)	INLET	31
WRITE(6,1168) (ALABL2(I),I=1,4)	INLET	32
1168 FORMAT(4X,4A10)	INLET	33
WRITE(6,1268)	INLET	34
1268 FORMAT(//,5X,(CONTROL CARDS))	INLET	35
C READ CONTROL CARDS	INLET	36
C	INLET	37
READ(5,1011) MINLET,NCYCLES,IPLOT,INT,ITABLE,C1,C2	INLET	38
WRITE(6,1012) MINLET,NCYCLES,IPLOT,INT,ITABLE,C1,C2	INLET	39
1011 FORMAT(5I10,2F10,5)	INLET	40
1012 FORMAT(1X,5I10,2F10,5)	INLET	41
C MINLET=THE NUMBER OF INLETS	INLET	42
C NCYCLES=NUMBER OF TIDAL CYCLES	INLET	43
C IPLOT (1 FOR A PLOT OF MEAN HYDRAULICS, 0 FOR NO PLOT)	INLET	44
C INT IS A PARAMETER DESCRIBING THE TYPE OF WEIGHING DESIRED	INLET	45
C INT=1 FOR FLOW WEIGHING TO ACHIEVE MINIMUM FRICTION	INLET	46
C INT=2 FOR WEIGHING FOR MINIMUM FRICTION WITH NO FLOW ACROSS CHANNELS	INLET	47
C INT=3 FOR EQUAL FLOW IN ALL GRIDS TO GIVE MAXIMUM FRICTION	INLET	48
C ITABLE=1 FOR A TABLE OF OUTPUT	INLET	49
C C1,C2 NC1=C2 * D. IF C1 AND C2 ARE ZERO THE MASHCH VALUES OF	INLET	50
C C1 =.03777 AND C2=.000667 ARE USED	INLET	51
IF(C1.EQ.0.0.AND.C2.EQ.0.0) C2=.000667	INLET	52
IF(C1.EQ.0.) C1=.03777	INLET	53
C	INLET	54

1	FORMAT(R110)	INLET	55
	READ(5,111) T,DPLT,A0,AN,BETA,ZETA,QINFLO	INLET	56
	WRITE(6,111) T,DPLT,A0,AN,BETA,ZETA,QINFLO	INLET	57
111	FORMAT(3F10.5,E10.4,F10.5)	INLET	58
C	TOTAL PERIOD: MRS (LATER CONVERTED TO SECONDS)	INLET	59
C	DELTA ESTIMATED TIME STEP, SEC	INLET	60
C	A0= SEA TIDAL AMPLITUDE, FT	INLET	61
C	AN= BAY AREA AT THE DATUM, SQUARE FEET	INLET	62
C	BETA= BAY AREA VARIATION PARAMETER ( D(AB)/D(AB) )	INLET	63
C	ZETA= CHANNEL SLOPE (D(Y)/D(X))	INLET	64
C	QINFLO= INFLOW INTO THE BAY FROM OTHER SOURCES (FT <sup>3</sup> /SEC)	INLET	65
C		INLET	66
	ENUN= NCYCLES*3600.	INLET	67
	IF(ZETA,LE,0.) ZETA=1, NZ=5	INLET	68
	NTR=0	INLET	69
C		INLET	70
C	READ IN INFORMATION OF EACH INLET	INLET	71
	DO 110 NI=1,NINLET	INLET	72
	UNIT=0.001	INLET	73
	RENIN=UNIT	INLET	74
	READ(5,1) IC,IS	INLET	75
C	IC= NUMBER OF CHANNELS	INLET	76
C	IS= NUMBER OF INLET CROSS-SECTIONS	INLET	77
	IF(IC,GT,7,OR,IS,GT,7) WRITE(6,1071)	INLET	78
1071	FORMAT(///,5X,100000 100 MANY GRIDS FOR DIMENSIONS(,///)	INLET	79
	ICM(NI)=IC	INLET	80
C	READ SECTION AREAS ( ONE CARD PER SECTION)	INLET	81
	DO 5 I=1,IS	INLET	82
5	READ(5,2) (A(I,J),J=1,IC)	INLET	83
2	FORMAT(10X,7F10.5)	INLET	84
C		INLET	85
C	READ SECTION WIDTHS (ONE CARD PER SECTION)	INLET	86
	DO 6 I=1,IS	INLET	87
6	READ(5,2) (B(I,J),J=1,IC)	INLET	88
C		INLET	89
	ICP=IC+1	INLET	90
	ISM=IS+1	INLET	91
C	READ LENGTHS (ONE MORE LENGTH PER CARD THAN CHANNELS)	INLET	92
C	( ONE LESS CARD THAN THE NUMBER OF SECTIONS)	INLET	93
	DO 7 I=1,ISM	INLET	94
7	READ(5,2) (L(I,J),J=1,ICP)	INLET	95
C		INLET	96
C	INITIALIZE VARIABLES TO BEGIN ITERATION	INLET	97
C	NUMBER OF GRIDS ALONG THE CHANNEL IS ONE LESS THAN THE NUMBER OF	INLET	98
C	CROSS-SECTIONS	INLET	99
00	ISM=IS	INLET	100
	ISE(NI)=IS	INLET	101
	ISM=IS	INLET	102
	WRITE(6,3070) NI	INLET	103
3070	FORMAT( //,5X,(SUMMARY OF INLET GRID CHARACTERISTICS(,//	INLET	104
	1 15X,(INLET NUMBER(,13)	INLET	105
	WRITE(6,1) IC,IS	INLET	106
	DO 10 I=1,IS	INLET	107



DO 11 J=1,IC	INLET	108
LENGTH(NI)=LENGTH(NT)+L(I,J)/FLOAT(IC)	INLET	109
A(I,J)=(A(I,J)+A(I+1,J))/2.	INLET	110
L(I,J)=(L(I,J)+L(I+1,J))/2.	INLET	111
B(I,J)=(B(I,J)+B(I+1,J))/2.	INLET	112
D(I,J)=(D(I,J)+D(I+1,J))/2.	INLET	113
N(I,J)=C1=C2=D(I,J)	INLET	114
LX(NI+I,J)=L(I,J)	INLET	115
BX(NI+I,J)=B(I,J)	INLET	116
DX(NI+I,J)=D(I,J)	INLET	117
NX(NI+I,J)=N(I,J)	INLET	118
NX(NI+I,J)=1./FLOAT(IC)	INLET	119
11 CONTINUE	INLET	120
WRITE(6,1207) I	INLET	121
1207 FORMAT(/,1X,(SECTION,13)	INLET	122
WRITE(6,1221) (NUMBER(I),I=1,IC)	INLET	123
1221 FORMAT(5X,(CHANNEL #,10I10,/) )	INLET	124
C PRINT A SUMMARY TABLE OF GEOMETRIES	INLET	125
WRITE(6,1971) (A(I,J),J=1,IC)	INLET	126
WRITE(6,1972) (R(I,J),J=1,IC)	INLET	127
WRITE(6,1973) (D(I,J),J=1,IC)	INLET	128
WRITE(6,1974) (L(I,J),J=1,IC)	INLET	129
WRITE(6,1975) (N(I,J),J=1,IC)	INLET	130
1971 FORMAT(5X,(AREA(FT <sup>2</sup> ),10F10,1)	INLET	131
1972 FORMAT(5X,(WIDTH(FT),10F10,1)	INLET	132
1973 FORMAT(5X,(DEPTH(FT),1X,10F10,2)	INLET	133
1974 FORMAT(5X,(LENGTH(FT),2X,10F10,1)	INLET	134
1975 FORMAT(5X,(N(10X,10F10,4)	INLET	135
10 CONTINUE	INLET	136
C FIND AREA AND WIDTH AT THE MINIMUM SECTION	INLET	137
AMINI(NI)=000.E+12	INLET	138
DO 100 J=1,IS	INLET	139
AA=0.	INLET	140
BB=0.	INLET	141
DO 100 J=1,IC	INLET	142
AAAA=AA(I,J)	INLET	143
100 BBBBB=BB(I,J)	INLET	144
IF(AA.GT.AMINI(NI)) GO TO 100	INLET	145
AMINI(NI)=AA	INLET	146
BB=BB(I,J)	INLET	147
100 CONTINUE	INLET	148
1110 CONTINUE	INLET	149
C ESTIMATE THE INLET-ORAY HELMHOLTZ PERIOD	INLET	150
CALL HELM(THELM,AB,CORL)	INLET	151
THTP=T/THELM	INLET	152
WRITE(6,201) T,THELM,THTP	INLET	153
201 FORMAT(1X,(FORCING PERIOD=,F7,2,( HOURS=,	INLET	154
1/,1X,(THELM(APPROX)=,F8,2,( HOURS=,	INLET	155
1 1X,(T/THELM=,10X,F8,2)	INLET	156
WRITE(6,1337) ((J+LNGTH(J),CORL(J)),J=1,NINLET)	INLET	157
1337 FORMAT( 1X,(INLET LENGTH ADDED LENGTH=, (,4X,12,1X,	INLET	158
1 F8,1,2X,F8,1))	INLET	159
T=T/3600.	INLET	160
CALL NRG5(END,DELT,NINLET,QINFLO,ITAB(,T)	INLET	161
DELT=END/FLOAT(NT)	INLET	162
DO 2260 NI=1,NINLET	INLET	163
NN=0	INLET	164
WRITE(6,2260) NI	INLET	165
2260 FORMAT(/,10X,(SUMMARY TABLE OF HYDRAULICS INLET,13)	INLET	166
(UNIT=,1)	INLET	167
CALL CRIT(NT,DELT,IUNIT,T,NCYCLES)	INLET	168
IF(TPLOT.EQ.1.AND.NI.EQ.1) CALL PLOTS(IAUF,1000,3)	INLET	169
IF(TPLOT.EQ.1) CALL GRPMC(ALANL1,ALANL2,DELT,IUNIT,NI)	INLET	170
IF(TPLOT.EQ.1.AND.NI.FO.NINLET) CALL PLNT(0,0,0,999)	INLET	171
2260 CONTINUE	INLET	172
STOP	INLET	173
END	INLET	174

SUBROUTINE RKGB(END,DFLT,NINLET,DINFLO,ITABLE,T)	INLET	175
C ROUTINE TO SOLVE A SET OF SIMULTANEOUS DIFFERENTIAL EQUATIONS	INLET	176
C ADAPTED FROM SCIENTIFIC SUBROUTINE PACKAGE, IBM, 1970	INLET	177
COMMON/NUM1/V(5),DERV(5),X,NT,INT,ZETA,NS	INLET	178
COMMON/NUM2/RNK(3,4)	INLET	179
DIMENSION AUX(4,5),A(4),B(4),C(4),PRMT(5),AMINI(3)	INLET	180
NDIM=NINLET+1	INLET	181
PRMT(1)=1.	INLET	182
PRMT(2)=END	INLET	183
PRMT(3)=DELT	INLET	184
PRMT(4)=.1	INLET	185
IF(T,GT,30000.) DELT=3000.	INLET	186
IF(T,LT,30000.) DELT=NT/9.	INLET	187
DO 1122 J=1,NINLET	INLET	188
V(J)=0.	INLET	189
1122 DERV(J)=0.,001	INLET	190
V(NDIM)=0.	INLET	191
DERV(NDIM)=1.0+FLOAT(NINLET)*0.001	INLET	192
DO 1 I=1,NDIM	INLET	193
1 AUX(I,1)=0.,0000000070DERV(I)	INLET	194
X=PRMT(1)	INLET	195
X=NDIM-PRMT(2)	INLET	196
N=PRMT(3)	INLET	197
PRMT(5)=0.	INLET	198
CALL SETQ(AMINI)	INLET	199
IF(NOT(X=0.)) 30.37.2	INLET	200
2 CONTINUE	INLET	201
A(1)=0.5	INLET	202
A(2)=0.2920932	INLET	203
A(3)=1.707107	INLET	204
A(4)=0.10000007	INLET	205
B(1)=2.	INLET	206
B(2)=1.	INLET	207
B(3)=1.	INLET	208
B(4)=2.	INLET	209
C(1)=0.5	INLET	210
C(2)=0.2920932	INLET	211
C(3)=1.707107	INLET	212
C(4)=0.5	INLET	213
DO 3 I=1,NDIM	INLET	214
AUX(I,1)=V(I)	INLET	215
AUX(2,1)=DERV(1)	INLET	216
AUX(3,1)=0.	INLET	217
AUX(4,1)=0.	INLET	218
3	INLET	219
NAMEH	INLET	220
IMLP=0	INLET	221
ISTEP=0	INLET	222
IF=0	INLET	223
4 CONTINUE	INLET	224
IF((X=0.)/X=END) 5H)7.6.3	INLET	225
5 CONTINUE	INLET	226
6 CONTINUE	INLET	227



MAXEND=X	INLET	228
IEND=1	INLET	229
7 CONTINUE	INLET	230
CALL SEA(MS,X)	INLET	231
CALL TPRTE(INLET,X,MS,QINPLO,V,AMINI,RNK,NT)	INLET	232
IFLAG1=X/DELY8	INLET	233
IF(IFLAG1.NE.,IFLAG2,AND,ITABLE,EO,1) CALL TABLE	INLET	234
IFLAG2=IFLAG1	INLET	235
IF(PRMT(5))40.0,40	INLET	236
8 CONTINUE	INLET	237
ITEST=0	INLET	238
9 CONTINUE	INLET	239
ISTEP=ISTEP+1	INLET	240
J=1	INLET	241
10 CONTINUE	INLET	242
AJ=AJ(J)	INLET	243
HJ=H(J)	INLET	244
CJ=C(J)	INLET	245
DO 11 I=1,NDIM	INLET	246
R1=H*DERV(I)	INLET	247
R2=AJ*(R1+QJ*AUX(4,I))	INLET	248
V(I)=V(I)+R2	INLET	249
R2=R2+H*QJ	INLET	250
11 AUX(4,I)=AUX(4,I)+R2=CJ*R1	INLET	251
IF(J=4)12,15,15	INLET	252
12 CONTINUE	INLET	253
J=J+1	INLET	254
IF(J=3)13,14,13	INLET	255
13 CONTINUE	INLET	256
X=X+0.5*H	INLET	257
14 CONTINUE	INLET	258
CALL BETEQ(AMINI)	INLET	259
GO TO 10	INLET	260
15 CONTINUE	INLET	261
IF(ITEST)16,16,20	INLET	262
16 CONTINUE	INLET	263
DO 17 I=1,NDIM	INLET	264
17 AUX(4,I)=V(I)	INLET	265
ITEST=1	INLET	266
ISTEP=ISTEP+ISTEP=2	INLET	267
18 CONTINUE	INLET	268
INLFI=INLFI+1	INLET	269
X=X+H	INLET	270
H=0.5*H	INLET	271
DO 19 I=1,NDIM	INLET	272
V(I)=AUX(1,I)	INLET	273
DERV(I)=AUX(2,I)	INLET	274
19 AUX(4,I)=AUX(3,I)	INLET	275
GO TO 9	INLET	276
20 CONTINUE	INLET	277
IMOD=ISTEP/2	INLET	278
IF(ISTEP=IMOD=IMOD)21,23,21	INLET	279
21 CONTINUE	INLET	280

	CALL SETEQ(AMINT)	INLET	281
	DO 22 I=1,NDIM	INLET	282
	AUX(5,I)=Y(I)	INLET	283
22	AUX(7,I)=DERV(I)	INLET	284
	GO TO 9	INLET	285
23	CONTINUE	INLET	286
	DELTA=0.	INLET	287
	DO 24 I=1,NDIM	INLET	288
24	DELTA=DELTA+AUX(6,I)*ABS(AUX(4,I)-Y(I))	INLET	289
	IF(DELTA=PRMT(4))26,26,25	INLET	290
25	CONTINUE	INLET	291
	IF(IMLF=10)26,36,36	INLET	292
26	CONTINUE	INLET	293
	DO 27 I=1,NDIM	INLET	294
27	AUX(4,I)=AUX(5,I)	INLET	295
	ISTEP=ISTEP+ISTEP=4	INLET	296
	X=X-H	INLET	297
	IFEND=0	INLET	298
	GO TO 1A	INLET	299
28	CONTINUE	INLET	300
	CALL SETEQ(AMINT)	INLET	301
	DO 29 I=1,NDIM	INLET	302
	AUX(1,I)=Y(I)	INLET	303
	AUX(2,I)=DERV(I)	INLET	304
	AUX(3,I)=AUX(6,I)	INLET	305
	Y(I)=AUX(5,I)	INLET	306
29	DERV(I)=AUX(7,I)	INLET	307
	CALL SEA(MS,X=H)	INLET	308
	CALL TPRMTE(NINLET,X=H,MS,QINFLO,Y,AMINI,RNK,NT)	INLET	309
	IFLAG1=(X=H)/DELTA	INLET	310
	IF(TFLAG1.NE,IFLAG2,AND,ITABLE,EQ,1) CALL TABLE	INLET	311
	IFLAG2=IFLAG1	INLET	312
	IF(PRMT(5))40,30,40	INLET	313
30	CONTINUE	INLET	314
	DO 31 I=1,NDIM	INLET	315
	Y(I)=AUX(1,I)	INLET	316
31	DERV(I)=AUX(2,I)	INLET	317
	INLET=IMLF	INLET	318
	IF(IEND)32,32,39	INLET	319
32	CONTINUE	INLET	320
	IMLF=IMLF+1	INLET	321
	ISTEP=ISTEP/2	INLET	322
	MS=H	INLET	323
	IF(IMLF)4,33,33	INLET	324
33	CONTINUE	INLET	325
	IMOD=ISTEP/2	INLET	326
	IF(ISTEP=IMOD=IMOD)4,34,4	INLET	327
34	CONTINUE	INLET	328
	IF(DELTA=0.02*PRMT(4))35,35,4	INLET	329
35	CONTINUE	INLET	330
	IMLF=IMLF+1	INLET	331
	ISTEP=ISTEP/2	INLET	332
	MS=H	INLET	333
	GO TO 4	INLET	334
36	CONTINUE	INLET	335
	IMLF=11	INLET	336
	CALL SETEQ(AMINT)	INLET	337
	GO TO 39	INLET	338
37	CONTINUE	INLET	339
	IMLF=12	INLET	340
	GO TO 39	INLET	341
38	CONTINUE	INLET	342
	IMLF=13	INLET	343
39	CONTINUE	INLET	344
	CALL SEA(MS,X)	INLET	345
	CALL TPRMTE(NINLET,X,MS,QINFLO,Y,AMINI,RNK,NT)	INLET	346
	IFLAG1=X/DELTA	INLET	347
	IF(TFLAG1.NE,IFLAG2,AND,ITABLE,EQ,1) CALL TABLE	INLET	348
	IFLAG2=IFLAG1	INLET	349
40	CONTINUE	INLET	350
	RETURN	INLET	351
	END	INLET	352



SUBROUTINE SETEQ(AMIN)	INLET	353
C ROUTINE TO SETUP THE EQUATIONS FOR THE RIGHT HAND SIDE OF THE EQUATIONS	INLET	354
C MOTION AND TO DETERMINE THE RANK OF THE TERMS IN THE EQUATION OF MOTIO	INLET	355
REAL L,LENGTH,LIN,LX,M,NX,LE	INLET	356
COMMON/NUM5/NI,G,MINLET,ICH(3),ISE(3),OR,L(7,7),B(7,7),D(7,7),	INLET	357
1 A(7,7),N(7,7),M(7,7),V(7,7),Q(7,7),HB,HB,M(7,7),IC,IS,AMINI(3),	INLET	358
1BMINI(3),LIN,QX(3),QINFLO,ARAY,LENGTH(3)	INLET	359
COMMON/NUM1/Y(5),DFRY(5),X,NT,INT,ZETA,MH	INLET	360
COMMON/NUM2/BX(1,7,7),DX(3,7,7),MX(3,7,7),LX(3,7,7),NX(3	INLET	361
1,7,7)	INLET	362
COMMON /NUM3/AO,T,ARY,BETA	INLET	363
COMMON/NUM4/RNK(3,4)	INLET	364
DIMENSION AMIN(3)	INLET	365
G=32,2	INLET	366
DO 220 NI=1,3	INLET	367
DO 119 I=1,4	INLET	368
119 RNK(NI,I)=0.	INLET	369
220 CONTINUE	INLET	370
CALL BEA(HB,X)	INLET	371
NHMBH	INLET	372
C FIND THE BAY AREA	INLET	373
HMBV(MINLET+1)	INLET	374
ARAY=ABV*(1.+BETA*HR)	INLET	375
QT=0.	INLET	376
C SET UP EQUATIONS FOR EACH INLET	INLET	377
DO 100 NI=1,MINLET	INLET	378
AMIN(NI)=999999999999.	INLET	379
QMBV(NI)	INLET	380
QT=QT+QD	INLET	381
IC=ICH(NI)	INLET	382
IS=ISE(NI)	INLET	383
LE=0.	INLET	384
DO 95 I=1,IS	INLET	385
DO 94 J=1,IC	INLET	386
N(I,J)=NX(NI,I,J)	INLET	387
L(I,J)=LX(NI,I,J)	INLET	388
LE=LE+L(I,J)/(FLOAT(IC))	INLET	389
94 B(I,J)=RX(NI,I,J)	INLET	390
95 CONTINUE	INLET	391
CALL LEVEL	INLET	392
AS=0.	INLET	393
AB=0.	INLET	394
AF=0.	INLET	395
DO 97 I=1,IS	INLET	396
AA=0.	INLET	397
DL=0.	INLET	398
DO 96 J=1,IC	INLET	399
DL=DL+L(I,J)/(FLOAT(IC)*LE)	INLET	400
D(I,J)=DX(NI,I,J)+N(I,J)	INLET	401
IF(D(I,J).LT.0.) D(I,J)=0.001	INLET	402
A(I,J)=B(I,J)*D(I,J)+H(I,J)*ABS(H(I,J))/(ZETA*FLOAT(IC))	INLET	403
IF(A(I,J).LT.0.) A(I,J)=0.001	INLET	404
IF(I.EQ.1) AS=AS+A(I,J)	INLET	405

	IF(I,EQ,IS) AB=AB+A(I,J)	INLET	406
96	AAAAA(I,J)	INLET	407
	IF(AA=LT,AMIN(NI)) AMIN(NI)=AA	INLET	408
97	AE=AE+DL/AA	INLET	409
	AMIN(NT)=AMIN(NI)	INLET	410
	AER1/AE	INLET	411
	IF(T=TEQ,1) CALL NT1	INLET	412
	IF(T=TEQ,2) CALL NT2	INLET	413
	IF(T=TEQ,3) CALL NT3	INLET	414
	DO 140 I=1,IS	INLET	415
	DO 139 J=1,IC	INLET	416
	MX(NI+I,J)=M(I,J)	INLET	417
139	MX(NI+I,J)=M(I,J)	INLET	418
140	CONTINUE	INLET	419
	RNK(NI,2)=AE/(2.0LE)*(1./(AB**2)-1./(AB**2))*QQ*QQ	INLET	420
	RNK(NI,3)=GG*AE/LE*(MB=MB)	INLET	421
	DO 85 I=1,IS	INLET	422
	AC=0.	INLET	423
	DO 84 J=1,IC	INLET	424
84	AC=AC+A(I,J)	INLET	425
	DO 83 J=1,IC	INLET	426
83	RNK(NI,4)=RNK(NT,4)+AE/(LE*AC)*GG*(I,J)**2*AUS(M(I,J)*QQ)*	INLET	427
	I*(I,J)*QQ/(2.20A*D(I,J)*0.33333*A(I,J)**2)*L(I,J)*B(I,J)	INLET	428
85	CONTINUE	INLET	429
	RNK(NI,1)=RNK(NI,2)+RNK(NI,3)+RNK(NI,4)	INLET	430
	DERV(NI)=RNK(NI,1)	INLET	431
	C FIND THE RELATIVE RANK OF TERMS, NORMALIZE BY THE LARGEST TERM,	INLET	432
	XMAX=0.	INLET	433
	DO 101 I=1,4	INLET	434
101	IF(ABS(RNK(NI,I)).GT,XMAX) XMAX=ABS(RNK(NI,I))	INLET	435
	DO 102 I=1,4	INLET	436
102	RNK(NI,I)=100.0*RNK(NI,I)/XMAX	INLET	437
100	CONTINUE	INLET	438
	DERV(NINLET+1)=GT/ARAY*QINFLO/ABAY	INLET	439
	RETURN	INLET	440
	END	INLET	441
	SUBROUTINE TPRYTE(NINLET,X,MB,QINFLO,Y,AMINI,RNK,NT)	INLET	442
C	SUBROUTINE TO WRITE HYDRAULIC INFORMATION ON TAPES	INLET	443
	DIMENSION RNK(3,4),Y(5),AMINI(3)	INLET	444
	HOURS=IX/3600.	INLET	445
	NT=NT+1	INLET	446
	DO 100 NI=1,NINLET	INLET	447
	IUNIT=NI+A	INLET	448
	V=V(NI)/AMINI(NI)	INLET	449
100	WRITE(IUNIT) HOURS,MB,QINFLO,Y(NINLET+1),V,V(NI),(RNK(NI,J),J=1,4)	INLET	450
	RETURN	INLET	451
	END	INLET	452



	SUBROUTINE LEVEL	INLET	453
C	THIS ROUTINE COMPUTES WATER LEVELS THROUGHOUT THE INLET ASSUMING LEVEL	INLET	454
C	ARE LINEAR FROM BAY TO SEA	INLET	455
	REAL L,LENGTH,LIN,LX,N,NX	INLET	456
	COMMON/NUMS/N,I,G,NINLET,ICH(3),ISE(3),QR,L(7,7),B(7,7),D(7,7),	INLET	457
	1 A(7,7),N(7,7),M(7,7),V(7,7),Q(7,7),MS,MB,M(7,7),IC,IS,AMINI(3),	INLET	458
	BMINI(3),LIN,QX(3),QINFLO,ABAY,LENGTH(3)	INLET	459
	DO 20 J=1,IC	INLET	460
	XL=0,	INLET	461
	DO 10 I=1,IS	INLET	462
10	XL=XL+L(I,J)	INLET	463
	XX=L(I,J)/2,	INLET	464
	M(I,J)=MS+(MB=MS)/XL*XX	INLET	465
	DO 11 I=2,IS	INLET	466
	XX=(L(I=1,J)+L(I,J))/2+XX	INLET	467
11	M(I,J)=MS+(MB=MS)/XL*XX	INLET	468
20	CONTINUE	INLET	469
	RETURN	INLET	470
	END	INLET	471
	SUBROUTINE SEA(MS,TIME)	INLET	472
C	THIS SUBROUTINE DETERMINES THE FORCING SEA LEVEL EITHER FROM	INLET	473
C	EQUAL-TIME-SERIES DATA (IF AVAILABLE) OR BY SINUSODIAL FORCING,	INLET	474
	COMMON /NUMS/AO,T,AB,RETA	INLET	475
	DIMENSION V(52)	INLET	476
	NN=NN+1	INLET	477
	IF(NN.NE.1) GO TO 10	INLET	478
	READ(5,1) TDEL,NPTS	INLET	479
1	FORMAT(3X,F0.2,XX,T3)	INLET	480
	TDEL=TDEL*60,	INLET	481
C	READ SEA LEVEL EQUAL TIME SERIES DATA THE FIRST TIME SEA IS CALLED	INLET	482
C	IF NPTS IS GREATER THAN 1	INLET	483
	IF(NPTS.GT.1) READ(5,2) (V(J),J=1,NPTS)	INLET	484
2	FORMAT(AF10.5)	INLET	485
	IF(NPTS.GT.1) WRITE(6,3) (V(J),J=1,NPTS)	INLET	486
3	FORMAT(3X,10F0.2)	INLET	487
	N1=NPTS+1	INLET	488
	N2=NPTS+2	INLET	489
	V(N1)=V(1)	INLET	490
	V(N2)=V(2)	INLET	491
10	IF(NPTS.LT.1) GO TO 100	INLET	492
C	INTERPOLATE IN TIME	INLET	493
	IT=TIME/T	INLET	494
	XT=TIME-IT*T	INLET	495
	J=XT/TDEL	INLET	496
	J=J+1	INLET	497
	MS=V(J)+((V(J+1)-V(J))*(XT-(J-1)*TDEL)/TDEL)	INLET	498
	RETURN	INLET	499
C	DETERMINE LEVEL IF SEA LEVEL FLUCTUATION IS SINUSODIAL	INLET	500
100	MS=AO* SIN(2.*3.14159*TIME/T)	INLET	501
	RETURN	INLET	502
	END	INLET	503

AD-A050 315

COASTAL ENGINEERING RESEARCH CENTER FORT BELVOIR VA  
A SPATIALLY INTEGRATED NUMERICAL MODEL OF INLET HYDRAULICS. (U)  
NOV 77 W N SEELIG, D L HARRIS

F/6 8/3

UNCLASSIFIED

CERC-6111-14

NL

2 OF 2

AD  
A050315



END  
DATE  
FILMED

3-78

DDC



SUBROUTINE HELM(THELM,AR,CORL)	INLET	504
C ESTIMATE THE INLET-RAV HELMHOLTZ PERIOD	INLET	505
C OF THE INLET/RAV SYSTEM (NEGLECT FRICTION)	INLET	506
REAL L,LENGTH,LIN,LX,N,NX	INLET	507
COMMON/NUMS/NI,G,NINLET,ICH(3),ISE(3),OR,L(7,7),O(7,7),D(7,7),	INLET	508
1 A(7,7),N(7,7),M(7,7),V(7,7),Q(7,7),MB,MB,M(7,7),IC,IS,AMINI(3),	INLET	509
IRMINI(3),LIN,OX(3),QINFLO,ARAY,LENGTH(3)	INLET	510
DIMENSION CORL(3)	INLET	511
C USE FIVE ITERATIONS TO OBTAIN THE ESTIMATE	INLET	512
DO 100 I=1,5	INLET	513
SUM=0.	INLET	514
DO 100 NN=1,NINLET	INLET	515
AMIN=AMINI(NN)	INLET	516
100 SUM=SUM+AMIN/(LENGTH(NN)*CORL(NN))	INLET	517
THELM=2.*3.14159* SQRT(AR/G)/ SQRT(SUM)	INLET	518
C ESTIMATE THE HELMHOLTZ PERIOD	INLET	519
DO 101 NN=1,NINLET	INLET	520
C ESTIMATE THE INLET LENGTH CORRECTION DUE TO RADIATION	INLET	521
101 CORL(NN)=IRMINI(NN)/3.14159*ALOG(3.14159*BMINI(NN)/( SQRT(	INLET	522
132.*AMINI(NN)/IRMINI(NN))*THELM)	INLET	523
1000 CONTINUE	INLET	524
C CONVERT THE HELMHOLTZ PERIOD TO HOURS	INLET	525
THELM=THELM/3600.	INLET	526
RETURN	INLET	527
END	INLET	528
 SUBROUTINE HTI	INLET	529
C THIS SUBROUTINE HEIGHTS THE FLOW IN EACH SECTION SO THAT FRICTION	INLET	530
C IN THAT SECTION IS MINIMIZED. THIS MEANS THAT AT EACH SECTION FLOW IS	INLET	531
C ADJUSTED TO REDISTRIBUTE ITSELF THROUGHOUT THE CHANNELS TO MINIMIZE FR	INLET	532
C HOWEVER, FLOW PERPENDICULAR TO THE CHANNELS IS ASSUMED TO BE SMALL AND	INLET	533
C FLOW IS NOT INCLUDED IN THE EQUATIONS OF MOTION. BY MINIMIZING FRICTI	INLET	534
C ROUTINE GIVES AN UPPER LIMIT FOR RAV LEVEL FLUCTUATIONS AND INLET VELO	INLET	535
REAL L,LENGTH,LIN,LX,N,NX	INLET	536
COMMON/NUMS/NI,G,NINLET,ICH(3),ISE(3),OR,L(7,7),O(7,7),D(7,7),	INLET	537
1 A(7,7),N(7,7),M(7,7),V(7,7),Q(7,7),MB,MB,M(7,7),IC,IS,AMINI(3),	INLET	538
IRMINI(3),LIN,OX(3),QINFLO,ARAY,LENGTH(3)	INLET	539
DIMENSION C(20)	INLET	540
DO 100 I=1,18	INLET	541
SUMC=0.	INLET	542
DO 50 J=1,IC	INLET	543
C(J)=A(1,J)**2*(D(1,J)**333)/	INLET	544
1 (N(1,J)**2*OX(NI)**2*O(1,J)*L(1,J))	INLET	545
50 SUMC=SUMC+C(J)	INLET	546
DO 60 J=1,IC	INLET	547
60 W(1,J)=C(J)/SUMC	INLET	548
100 CONTINUE	INLET	549
RETURN	INLET	550
END	INLET	551

```

SUBROUTINE M72
C ROUTINE TO DETERMINE THE GRID *FIGHTING FUNCTION ASSUMING THAT
C FLOW IN A GIVEN CHANNEL IS THE SAME ALONG THE ENTIRE CHANNEL
C FLOW IS DISTRIBUTED IN CHANNELS TO GIVE A MINIMUM TOTAL FRICTION
C FRICTION IN THIS ROUTINE WILL BE SLIGHTLY HIGHER THAN IN M71 AND THE
C IN THIS SYSTEM IS CONSISTANT WITH THE EQUATIONS OF MOTION.
REAL L,LENGTH,LIN,LX,N,NX
COMMON/NUMS,N1,0,NINLET,ICH(3),ISF(3),OR,L(7,7),B(7,7),D(7,7),
1 A(7,7),N(7,7),M(7,7),V(7,7),Q(7,7),MB,MB,M(7,7),IC,IS,AMINI(3),
1BMINI(3),LIN,OX(3),QINFLO,ABAY,LENGTH(3)
DIMENSION C(20)
SUMC=0.
DO 100 I=1,IC
C(I)=0.
DO 50 J=1,IS
C(I)=C(I)+(N(J,I)*2*OX(NI)*2*(B(J,I)*L(J,I))/
1 (A(J,I)*2+(D(J,I)*.333333))
100 SUMC=SUMC+C(I)
DO 70 J=1,IS
DO 40 I=1,IC
40 W(J,I)=C(I)/SUMC
70 CONTINUE
RETURN
END

```

```

INLET 552
INLET 553
INLET 554
INLET 555
INLET 556
INLET 557
INLET 558
INLET 559
INLET 560
INLET 561
INLET 562
INLET 563
INLET 564
INLET 565
INLET 566
INLET 567
INLET 568
INLET 569
INLET 570
INLET 571
INLET 572
INLET 573
INLET 574
INLET 575
INLET 576

```

```

SUBROUTINE M73
C THIS ROUTINE ASSUMES THAT DISCHARGE IS EQUALLY DISTRIBUTED THROUGHOUT
C THE INLET GRID SYSTEM. IN GENERAL THIS WILL NOT BE TRUE BECAUSE IT IS
C DIFFICULT TO ACCURATELY DRAW THIS TYPE OF GRID BY EYE AND FLOW DISTRUB
C CHANGES WITH TIME IN MOST INLETS. THIS ROUTINE IS USEFUL IN GIVING AN
C VELOCITIES AND RAY LEVEL FLUCTUATIONS.
C GRIDS WITH DEPTHS LT 0.01 FOOT ARE ASSUMED TO HAVE NO FLOW
REAL L,LENGTH,LIN,LX,N,NX
COMMON/NUMS,N1,0,NINLET,ICH(3),ISE(3),OR,L(7,7),B(7,7),D(7,7),
1 A(7,7),N(7,7),M(7,7),V(7,7),Q(7,7),MB,MB,M(7,7),IC,IS,AMINI(3),
1BMINI(3),LIN,OX(3),QINFLO,ABAY,LENGTH(3)
DO 2 I=1,IS
IX=IC
DO 1 J=1,IC
1 IF(D(I,J).LT.0.01) XXX=1.
IF(XXX.EQ.1) WRITE(6,100) M73 IS
100 FORMAT(//,5X,1 ERROR == INLET HAS DRIED UP AS INDICATED IN M73,1//
1 5X, (INLET=I,1=J) SECTION=I,1=J,1//)
IF(XXX.EQ.1) STOP
DO 3 J=1,IC
W(I,J)=1./X
3 IF(D(I,J).LT.0.01) W(I,J)=0.
2 CONTINUE
RETURN
END

```

```

INLET 577
INLET 578
INLET 579
INLET 580
INLET 581
INLET 582
INLET 583
INLET 584
INLET 585
INLET 586
INLET 587
INLET 588
INLET 589
INLET 590
INLET 591
INLET 592
INLET 593
INLET 594
INLET 595
INLET 596
INLET 597
INLET 598
INLET 599
INLET 600
INLET 601

```





	SUBROUTINE CRIT(NT,NELT,IUNIT,T,NCYCLES)	INLET	650
C	SUBROUTINE CRIT COMPARES 3 CONSECUTIVE FUNCTION POINTS	INLET	651
C	AND WRITES MIDDLE POINT IF IT IS A CRITICAL POINT	INLET	652
		INLET	653
	DIMENSION P(3,5),MARK(5),TERM(4)	INLET	654
	DATA MARKA/1H /, MARKB/1H0/	INLET	655
	RENTND IUNIT	INLET	656
	NLINE=0	INLET	657
	TFOT/3600.	INLET	658
	WRITE(6,1009)	INLET	659
	DO 1 N=1,2	INLET	660
1	READ(IUNIT) X=(P(N,J),J=1,5),(TERM(I),I=1,4)	INLET	661
	DO 100 N=3,NT	INLET	662
	READ(IUNIT) X=(P(3,J),J=1,5),(TERM(I),I=1,4)	INLET	663
	IF(X,LT,-1.0E+10) GO TO 101	INLET	664
	IOUT=0	INLET	665
	DO 2020 IA = 1, 5	INLET	666
	MARK(IA) = MARKA	INLET	667
2012	IF (P(2,IA) = P(1,IA)) 2012, 2020, 2014	INLET	668
2014	IF (P(3,IA) = P(2,IA)) 2020, 2015, 2015	INLET	669
2015	IF (P(3,IA) = P(2,IA)) 2015, 2015, 2020	INLET	670
	CRITICAL POINT VALUE FOUND	INLET	671
2015	IOUT = 1	INLET	672
	MARK(IA) = MARKB	INLET	673
	IF(IA.EQ.1.AND.P(2,IA).GT.0.) MSHOF(2,IA)	INLET	674
	IF(IA.EQ.1.AND.P(2,IA).GT.0.) T1=H	INLET	675
	IF(IA.EQ.1.AND.P(2,IA).LT.0.) MSLUF(2,IA)	INLET	676
	IF(IA.EQ.1.AND.P(2,IA).LT.0.) T2=H	INLET	677
	IF(IA.EQ.3.AND.P(3,IA).GT.0.) MSHOF(3,IA)	INLET	678
	IF(IA.EQ.3.AND.P(3,IA).GT.0.) T3=H	INLET	679
	IF(IA.EQ.3.AND.P(3,IA).LT.0.) MSLUF(3,IA)	INLET	680
	IF(IA.EQ.3.AND.P(3,IA).LT.0.) T4=H	INLET	681
	IF(IA.EQ.4.AND.P(2,IA).LT.0.) VEOF(2,IA)	INLET	682
	IF(IA.EQ.4.AND.P(2,IA).GT.0.) VEOF(2,IA)	INLET	683
2020	CONTINUE	INLET	684
	DO 2025 IA = 1, 5	INLET	685
	P(1,IA) = P(2,IA)	INLET	686
2025	P(2,IA) = P(3,IA)	INLET	687
	IF (IOUT.EQ.0) GO TO 100	INLET	688
C	IF(X,LT,(NCYCLES-2)*TF) GO TO 100	INLET	689
	NLINE=NLINE+1	INLET	690
	IF(NLINE.GT.150) GO TO 100	INLET	691
	WRITE(6,2101) X=(P(1,IA),MARK(IA),IA,1,5)	INLET	692
100	CONTINUE	INLET	693
101	CONT	INLET	694
	AMPH=MSH/MSL	INLET	695
	AMPL=MSL/MSL	INLET	696
	PHM=ABS(T3-T1)*360./TF	INLET	697
	PHL=ABS(T4-T2)*360./TF	INLET	698
	WRITE(6,1011) AMPH,PHM,VF,AMPL,PHL,VE	INLET	699
	WRITE(6,1111) TF	INLET	700
1111	FORMAT( 5X,(TF=1.P7,2)	INLET	701
	RETURN	INLET	702
2101	FORMAT (2F8.3,A1,=3PF8.3,A1,2(0PF7.3,A1),	INLET	703
	3PF8.3, A1, 2(F8.3, A1))	INLET	704
1000	FORMAT(4X,AMTIME,5X,2PHM,4X,6MINFLOW,5X,2PHM,	INLET	705
	1 5X,3MVEL,7X,1H0./,5X,3MHRB,5X,2MFT,5X,2MKCFB,	INLET	706
	1 6X,2MFT,5X,3MPPB,4X,2MKCFB,7)	INLET	707
1011	FORMAT(//,1X,10 CRITICAL POINT VALUE(//,1X,	INLET	708
	1 (WAVE PROPAGATION(//,1X,(AB/AO(1X,(PHASE LAB(DEG) MAX VEL(	INLET	709
	1//,2X,(HIGH WATER(,2X,3F10.4//,	INLET	710
	1 2X,(LOW WATER (,2X,3F10.4)	INLET	711
	END	INLET	712



	SUBROUTINE READIN (X,V,YFAC,XFAC,X0,XF,INDC,KK,LN,IUNIT)	INLET	713
C	SUBROUTINE TO READ SOLUTION TABULATION FROM FILE	INLET	714
C		INLET	715
	DIMENSION V(9), VFAC(9)	INLET	716
	DTSM=501./60.	INLET	717
	READ (IUNIT) X, V	INLET	718
	IF(X,LT=1,E=10) KK=2	INLET	719
	INDC = 0	INLET	720
	IF (KK = 1) 10, 10, 50	INLET	721
10	IF (X0 = X = 0T5) 20, 50, 50	INLET	722
20	IF (X = XF = 0T5) 30, 25, 25	INLET	723
25	KK = 2	INLET	724
	GO TO 50	INLET	725
30	INDC = 1	INLET	726
	X = XFAC(X = X0)	INLET	727
	V(LN) = VFAC(LN)*V(LN)	INLET	728
50	RETURN	INLET	729
	END	INLET	730
	SUBROUTINE GRPHC(ALABL1,ALABL2,DELT,IUNIT,N1)	INLET	731
C		INLET	732
C	SUBROUTINE GRPHC WRITES PLOTTER TAPE FOR GRAPHICAL	INLET	733
C	OUTPUT OF SOLUTION	INLET	734
C		INLET	735
	DIMENSION BL(2), ISYM(5)	INLET	736
	DIMENSION VLABELL(3),ALEGN(3,6),ALABL1(4),ALABL2(4),SYM(3),Y(9),YFA	INLET	737
	IC(9),XX(2000),VV(2000),TT(9,2)	INLET	738
	DATA VLABELL/10HHEIGHTS, V,10HVELOCITIES,0.8H=FT, FPD/	INLET	739
	DATA ALEGN/10HFLOW (KCFB,10H) .3H .10HINLET VELO,10HCITY	INLET	740
	1 (FT/0.3HSEC),10HDAY LEVEL(,10HFT) .3H .10HINFLOW .10H	INLET	741
	2 .3H .10HOCEAN LEVE,10H(FT) .3H .10HLEGEND .10H	INLET	742
	3 .3H /	INLET	743
	DATA BL/10HOBERVED B,10HAY TIDE /	INLET	744
	DATA ISYM/5,4,3,2,1/	INLET	745
	DATA TT(6,1)/10HTEMPORAL A/	INLET	746
	DATA TT(6,2)/10HCCCEL /	INLET	747
	DATA TT(7,1)/10HCONVECTIVE/	INLET	748
	DATA TT(7,2)/10H ACC /	INLET	749
	DATA TT(8,1)/10HMPRESSURE H/	INLET	750
	DATA TT(8,2)/10HEAD /	INLET	751
	DATA TT(9,1)/10HBOTTOM STR/	INLET	752
	DATA TT(9,2)/10HESS /	INLET	753
C		INLET	754
C	READ INFORMATION TO DIRECT PLOTTING	INLET	755
C		INLET	756
C	FIRST CARD	INLET	757
C	X0 = STARTING TIME OF PLOT (HRS)	INLET	758
C	XF = ENDING TIME OF PLOT (HRS)	INLET	759
C	SCALEX = TIME AXIS SCALE IN HOURS PER INCH	INLET	760
C	VLO = MINIMUM VALUE OF TIDAL HEIGHTS (FT)	INLET	761
C	VL = OVERALL HEIGHT OF PLOT (INCHES)	INLET	762
C	VLSCAL = SCALE OF TIDAL HEIGHTS (FT/INCH)	INLET	763
C	VRO = MINIMUM VALUE OF FLOWS (THOUSANDS OF CUBIC FEET PER SECOND)	INLET	764
C	VRSCL = SCALE OF FLOW ( THOUSANDS OF CUBIC FEET PER SECOND/INCH)	INLET	765
C		INLET	766
C	CARD 2	INLET	767
C	VVO = MINIMUM VELOCITY (FT/SEC)	INLET	768
C	VVSCAL = SCALE OF VELOCITY (FEET PER SECOND/INCH)	INLET	769
C	SCAL = SCALE FACTOR FOR TOTAL PLOT SIZE	INLET	770
C	IG = NOT EQUAL TO ZERO FOR A PLOT OF INLET DISCHARGE	INLET	771
C		INLET	772

IF(NI.EQ.1)	INLET	773
1 READ (5,2001) X0,XF,SCALX,YL0,YL,YLSCAL,YR0,YRSCAL,YV0,YVSCAL,	INLET	774
1 SCALE,10	INLET	775
2001 FORMAT(AP10.5,/.3F10.5,I10)	INLET	776
WRITE(6,2002) X0,XF,SCALX,YL0,YL,YLSCAL,YR0,YRSCAL,YV0,YVSCAL,	INLET	777
1 SCALE,10	INLET	778
2002 FORMAT(///,5X,(PLOT INFORMATION:,,	INLET	779
1'X,0F10.5,/.1X,3F10.5,I10)	INLET	780
C DETERMINE SYMBOL SPACING	INLET	781
LINTYP=25*SCALX/(DELT/3000,)	INLET	782
WRITE(6,1215) LINTYP	INLET	783
1215 FORMAT(1X,(LINTYP(1,6)	INLET	784
C	INLET	785
C PLOT LEGEND	INLET	786
C	INLET	787
CALL SYMBOL(1,0,YL/2,0,0,20,0,MLEGEND,0,0,6)	INLET	788
DO 20 LN = 1, 5	INLET	789
INDX = 0	INLET	790
YPR=YL/2,0,0,0,0,0,2	INLET	791
LLN=ISYM(LN)	INLET	792
CALL SYMBOL(0,0,YPR,0,0,0,10,LLN,0,0,1)	INLET	793
SYM(1) = ALEGN(1,LN)	INLET	794
SYM(2) = ALEGN(2,LN)	INLET	795
SYM(3) = ALEGN(3,LN)	INLET	796
CALL SYMBOL(.4,YPR,0,1,SYM,0,0,23)	INLET	797
20 CONTINUE	INLET	798
C PLOT TITLE	INLET	799
CALL SYMBOL(3,5,YL/2,-1,0,21,ALABL1,0,0,32)	INLET	800
CALL SYMBOL(3,5,YL/2,-1,0,21,ALABL2,0,0,32)	INLET	801
C PLOT AXES	INLET	802
YLO=YL/2,YLSCAL	INLET	803
CALL AXIS(0,0,YL/2,0,10,VELOCITY, FT/SEC,10,YL,90,0,YV0	INLET	804
1,YVSCAL)	INLET	805
CALL AXIS(0,0,YL/2,0,10,HHEIGHTS, FT,11,YL,90,0,YL0,YLSCAL)	INLET	806
CALL AXIS(0,0,YL/2,0,0,TIME, HRS,0,(XF=X0)/SCALX,0,0,0,SCALX)	INLET	807
IF(TD.NE.0)	INLET	808
1CALL AXIS((XF-X0)/SCALX,YL/2,0,10,FLOW, KCFB,0,10,YL,90,0,YL/2,0,YR	INLET	809
10CAL,YRSCAL)	INLET	810
IF(TD.EQ.0) CALL PLOT((XF-X0)/SCALX,YL/2,0,3)	INLET	811
IF(TD.EQ.0) CALL PLOT((XF-X0)/SCALX,YL/2,0,2)	INLET	812
CALL PLOT((XF-X0)/SCALX,YL/2,0,3)	INLET	813
CALL PLOT(0,0,YL/2,0,2)	INLET	814
YFAC(1) = 1./YLSCAL	INLET	815
YFAC(2) = 0.001/YRSCAL	INLET	816
YFAC(3) = YFAC(1)	INLET	817
YFAC(4) = 1./YVSCAL	INLET	818
YFAC(5) = YFAC(2)	INLET	819
DO 1234 I=0,0	INLET	820
1234 YFAC(I)=0.003	INLET	821
XFAC = 1./SCALX	INLET	822
DO 85 I = 1, 0	INLET	823
C IF I=0 DO NOT PLOT DISCHARGE	INLET	824
IF(TD.EQ.0.AND.I.EQ.0) GO TO 85	INLET	825
COM=YL/2,0,(I=5)0,0	INLET	826
CALL PLOT(0,0,0,3)	INLET	827
NN = 1	INLET	828
ISUR=0	INLET	829
REWIND UNIT	INLET	830



	INDX = 0	INLET	031
05	CALL READIN (X,V,YFAC,XFAC,X0,XF,INDC,KK,I,IUNIT)	INLET	032
	GO TO (70, 80), KK	INLET	033
70	IF(INDC,LE,0) GO TO 65	INLET	034
72	ISUR=IS(10+1)	INLET	035
	IF(ISUR,GE,1998) ISUR=1998	INLET	036
	XX(ISUR)=X	INLET	037
	VY(ISUR)=Y(I)	INLET	038
	IF(1,GT,5) VY(ISUR)=VY(ISUR)+COR	INLET	039
	IF(ISUR,EQ,1998) GO TO 80	INLET	040
	GO TO 65	INLET	041
80	XX(ISUR+1)=0.	INLET	042
	XX(ISUR+2)=1.0	INLET	043
	VY(ISUR+1)=0.	INLET	044
	VY(ISUR+2)=1.	INLET	045
	C PLOT CURVES ( DO NOT PLOT IF EQUAL TO ZERO THROUGHOUT)	INLET	046
	IF(VY(ISUR+2),EQ,0.0,AND,	INLET	047
	1 VY(ISUR+1),EQ,0.0,AND,VY(ISUR),EQ,0.0) GO TO 85	INLET	048
	IF(1,GT,5) GO TO 885	INLET	049
	CALL LINE(XX,VY,ISUR+1,LINTVP,I)	INLET	050
	GO TO 85	INLET	051
885	CALL LINE(XX,VY,ISUR+1,0.0)	INLET	052
	CALL PLOT((X=X0)/SCALX+COR,3)	INLET	053
	CALL PLOT(0.,COR,2)	INLET	054
	SYM(1)=YT(1,1)	INLET	055
	SYM(2)=YT(1,2)	INLET	056
	CALL SYMBOL(-2,2,COR,0.1,SYM,0.,20)	INLET	057
05	CONTINUE	INLET	058
	C READ PHOTOTYPE RAY TIDE (DATA STARTS AT BEGINNING OF PLOT, SAME DATUM)	INLET	059
	IF(NI,NE,1) GO TO 2010	INLET	060
	READ(5,1) TDEL,NPTS	INLET	061
1	FORMAT(34X,F6.2,6X,13)	INLET	062
	IF(NPTS,LT,2) GO TO 2010	INLET	063
	IF(NPTS,GT,1) READ(5,2) (VY(J),J=1,NPTS)	INLET	064
2	FORMAT(A10,5)	INLET	065
	XX(NPTS+1)=0.	INLET	066
	XX(NPTS+2)=1.	INLET	067
	VY(NPTS+1)=0.	INLET	068
	VY(NPTS+2)=1.	INLET	069
	DO 3 J=1,NPTS	INLET	070
	VY(J)=VY(J)+YFAC(1)	INLET	071
3	XX(J)=(TDEL/60.)*XFAC*(J-1)	INLET	072
	CALL PLOT(XX(1),VY(1),3)	INLET	073
	CALL LINE(XX,VY,NPTS+1,0.0)	INLET	074
	CALL PLOT(XX(NPTS/2),VY(NPTS/2),3)	INLET	075
	CALL PLOT(XX(NPTS/2),VY(NPTS/2)+.75,2)	INLET	076
	CALL SYMBOL(XX(NPTS/2)+.1,VY(NPTS/2)+.75,.1,0L,0.,17)	INLET	077
2010	CALL PLOT((X=X0)/SCALX+4.,0.,3)	INLET	078
	RETURN	INLET	079
	END	INLET	080

<p>Seelig, William N.</p> <p>A spatially integrated numerical model of inlet hydraulics / by William N. Seelig, D. Lee Harris...[et al.] - Fort Belvoir, Va. : U.S. Coastal Engineering Research Center ; Springfield, Va. : available from National Technical Information Service, 1977.</p> <p>101 p. : ill. (GITI report 14)</p> <p>Bibliography: p. 55.</p> <p>This report discusses the development of a simple numerical model for the prediction of coastal inlet velocities, discharge, and resulting bay level fluctuations. The model is a time-marching model that simultaneously solves the area-averaged momentum equation for the inlet and the continuity equation for the bay.</p> <p>1. Tidal inlets. 2. Tidal hydraulics. 3. Water levels. 4. Computer programs. 5. Numerical models. I. Title. II. Harris, D. Lee, joint author. III. Series: U.S. Army. Corps of Engineers. GITI report 14.</p> <p>CB454 .I5 .U581r no. 14 551.4</p>	<p>Seelig, William N.</p> <p>A spatially integrated numerical model of inlet hydraulics / by William N. Seelig, D. Lee Harris...[et al.] - Fort Belvoir, Va. : U.S. Coastal Engineering Research Center ; Springfield, Va. : available from National Technical Information Service, 1977.</p> <p>101 p. : ill. (GITI report 14)</p> <p>Bibliography: p. 55.</p> <p>This report discusses the development of a simple numerical model for the prediction of coastal inlet velocities, discharge, and resulting bay level fluctuations. The model is a time-marching model that simultaneously solves the area-averaged momentum equation for the inlet and the continuity equation for the bay.</p> <p>1. Tidal inlets. 2. Tidal hydraulics. 3. Water levels. 4. Computer programs. 5. Numerical models. I. Title. II. Harris, D. Lee, joint author. III. Series: U.S. Army. Corps of Engineers. GITI report 14.</p> <p>CB454 .I5 .U581r no. 14 551.4</p>
<p>Seelig, William N.</p> <p>A spatially integrated numerical model of inlet hydraulics / by William N. Seelig, D. Lee Harris...[et al.] - Fort Belvoir, Va. : U.S. Coastal Engineering Research Center ; Springfield, Va. : available from National Technical Information Service, 1977.</p> <p>101 p. : ill. (GITI report 14)</p> <p>Bibliography: p. 55.</p> <p>This report discusses the development of a simple numerical model for the prediction of coastal inlet velocities, discharge, and resulting bay level fluctuations. The model is a time-marching model that simultaneously solves the area-averaged momentum equation for the inlet and the continuity equation for the bay.</p> <p>1. Tidal inlets. 2. Tidal hydraulics. 3. Water levels. 4. Computer programs. 5. Numerical models. I. Title. II. Harris, D. Lee, joint author. III. Series: U.S. Army. Corps of Engineers. GITI report 14.</p> <p>CB454 .I5 .U581r no. 14 551.4</p>	<p>Seelig, William N.</p> <p>A spatially integrated numerical model of inlet hydraulics / by William N. Seelig, D. Lee Harris...[et al.] - Fort Belvoir, Va. : U.S. Coastal Engineering Research Center ; Springfield, Va. : available from National Technical Information Service, 1977.</p> <p>101 p. : ill. (GITI report 14)</p> <p>Bibliography: p. 55.</p> <p>This report discusses the development of a simple numerical model for the prediction of coastal inlet velocities, discharge, and resulting bay level fluctuations. The model is a time-marching model that simultaneously solves the area-averaged momentum equation for the inlet and the continuity equation for the bay.</p> <p>1. Tidal inlets. 2. Tidal hydraulics. 3. Water levels. 4. Computer programs. 5. Numerical models. I. Title. II. Harris, D. Lee, joint author. III. Series: U.S. Army. Corps of Engineers. GITI report 14.</p> <p>CB454 .I5 .U581r no. 14 551.4</p>



<p>Seelig, William N.</p> <p>A spatially integrated numerical model of inlet hydraulics / by William N. Seelig, D. Lee Harris...[et al.] - Fort Belvoir, Va. : U.S. Coastal Engineering Research Center ; Springfield, Va. : available from National Technical Information Service, 1977.</p> <p>101 p. : ill. (GITI report 14)</p> <p>Bibliography: p. 55.</p> <p>This report discusses the development of a simple numerical model for the prediction of coastal inlet velocities, discharge, and resulting bay level fluctuations. The model is a time-marching model that simultaneously solves the area-averaged momentum equation for the inlet and the continuity equation for the bay.</p> <p>1. Tidal inlets. 2. Tidal hydraulics. 3. Water levels. 4. Computer programs. 5. Numerical models. I. Title. II. Harris, D. Lee, joint author. III. Series: U.S. Army. Corps of Engineers. GITI report 14.</p> <p>GB454 .15 .U581r no. 14 551.4</p>	<p>Seelig, William N.</p> <p>A spatially integrated numerical model of inlet hydraulics / by William N. Seelig, D. Lee Harris...[et al.] - Fort Belvoir, Va. : U.S. Coastal Engineering Research Center ; Springfield, Va. : available from National Technical Information Service, 1977.</p> <p>101 p. : ill. (GITI report 14)</p> <p>Bibliography: p. 55.</p> <p>This report discusses the development of a simple numerical model for the prediction of coastal inlet velocities, discharge, and resulting bay level fluctuations. The model is a time-marching model that simultaneously solves the area-averaged momentum equation for the inlet and the continuity equation for the bay.</p> <p>1. Tidal inlets. 2. Tidal hydraulics. 3. Water levels. 4. Computer programs. 5. Numerical models. I. Title. II. Harris, D. Lee, joint author. III. Series: U.S. Army. Corps of Engineers. GITI report 14.</p> <p>GB454 .15 .U581r no. 14 551.4</p>
<p>Seelig, William N.</p> <p>A spatially integrated numerical model of inlet hydraulics / by William N. Seelig, D. Lee Harris...[et al.] - Fort Belvoir, Va. : U.S. Coastal Engineering Research Center ; Springfield, Va. : available from National Technical Information Service, 1977.</p> <p>101 p. : ill. (GITI report 14)</p> <p>Bibliography: p. 55.</p> <p>This report discusses the development of a simple numerical model for the prediction of coastal inlet velocities, discharge, and resulting bay level fluctuations. The model is a time-marching model that simultaneously solves the area-averaged momentum equation for the inlet and the continuity equation for the bay.</p> <p>1. Tidal inlets. 2. Tidal hydraulics. 3. Water levels. 4. Computer programs. 5. Numerical models. I. Title. II. Harris, D. Lee, joint author. III. Series: U.S. Army. Corps of Engineers. GITI report 14.</p> <p>GB454 .15 .U581r no. 14 551.4</p>	<p>Seelig, William N.</p> <p>A spatially integrated numerical model of inlet hydraulics / by William N. Seelig, D. Lee Harris...[et al.] - Fort Belvoir, Va. : U.S. Coastal Engineering Research Center ; Springfield, Va. : available from National Technical Information Service, 1977.</p> <p>101 p. : ill. (GITI report 14)</p> <p>Bibliography: p. 55.</p> <p>This report discusses the development of a simple numerical model for the prediction of coastal inlet velocities, discharge, and resulting bay level fluctuations. The model is a time-marching model that simultaneously solves the area-averaged momentum equation for the inlet and the continuity equation for the bay.</p> <p>1. Tidal inlets. 2. Tidal hydraulics. 3. Water levels. 4. Computer programs. 5. Numerical models. I. Title. II. Harris, D. Lee, joint author. III. Series: U.S. Army. Corps of Engineers. GITI report 14.</p> <p>GB454 .15 .U581r no. 14 551.4</p>